FATIGUE LIFE PREDICTIONS FOR ASPHALT CONCRETE SUBJECTED TO MULTIPLE AXLE LOADINGS

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ABSTRACT

The Load Equivalency Factor (LEF) and the Truck Factor (TF) are defined as the relative damage of an axle group or a truck to that of a standard axle. In the mechanistic approach, the fatigue damage caused by a given axle configuration is calculated using fatigue equations derived from single haversine or continuous sinusoidal loading pulses. In this paper, the fatigue damage of an asphalt mixture under different axle groups and truck configurations was determined directly from the indirect tensile cyclic load test by using load pulses that are equivalent to the transverse response due to the passage of an entire axle group or truck. In addition, the fatigue damage was obtained for different pavement structures using the SAPSI-M computer program and compared with laboratory results. The pavement fatigue damage and the LEFs/TFs were calculated using three different methods: peak strains, peak-midway strains, and dissipated energy. The results reveal that, in general, the LEFs/TFs based on the peak-midway strain method agree reasonably well with those from the dissipated energy method. On the other hand, the peak strain method overestimates the transverse LEF values.

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ABSTRACT

The Load Equivalency Factor (LEF) and the Truck Factor (TF) are defined as the relative damage of an axle group or a truck to that of a standard axle. In the mechanistic approach, the fatigue damage caused by a given axle configuration is calculated using fatigue equations derived from single haversine or continuous sinusoidal loading pulses. In this paper, the fatigue damage of an asphalt mixture under different axle groups and truck configurations was determined directly from the indirect tensile cyclic load test by using load pulses that are equivalent to the transverse response due to the passage of an entire axle group or truck. In addition, the fatigue damage was obtained for different pavement structures using the SAPSI-M computer program and compared with laboratory results. The pavement fatigue damage and the LEFs/TFs were calculated using three different methods: peak strains, peak-midway strains, and dissipated energy. The results reveal that, in general, the LEFs/TFs based on the peak-midway strain method agree reasonably well with those from the dissipated energy method. On the other hand, the peak strain method overestimates the transverse LEF values.

1. INTRODUCTION

Load Equivalency Factor (LEF) is defined as the damage of the pavement caused by a given axle relative to the standard 80 kN (18 kip) axle, and has played an important role in mechanistic pavement design. According to Miner's hypothesis, damage is the inverse of the number of load repetitions until the failure of the structure. That is,

$$Damage = \frac{1}{N_f}$$

where N_f is the number of load repetitions until failure. In pavement engineering literature, many researchers have tested the material under fatigue and came up with equations that may be used to estimate the N_f with initial strain or dissipated energy (e.g., Monismith et al, 1994). These equations have the form,

$$N_f = \alpha_1 \cdot \varepsilon_o^{\beta_1}$$
$$N_f = \alpha_2 \cdot w_o^{\beta_2}$$

where $\varepsilon_o \equiv \text{initial strain}$

 $w_o \equiv$ initial dissipated energy $\alpha_i, \beta_i \equiv$ constants

The equations are typically based on single loading or continuous sinusoidal loading. However, when a vehicle travels over the pavement, a given point in the pavement is subjected to multiple pulses depending on the axle configuration. In addition, there are two directional responses; transverse and longitudinal. Thus the fatigue equations could be developed based on transverse or longitudinal responses (tension or compression-tension). Using a fatigue equation that is developed from transverse responses to estimate the fatigue damage in the longitudinal direction may be inadequate.

The objectives of this paper are to: (1) investigate the fatigue damage caused by multiple axle groups and different truck configurations, and (2) compare different methods of predicting fatigue damage. Because of testing limitations, fatigue equations for longitudinal response (compression-tension loading) could not be developed. Therefore the results presented in this paper are limited to transverse response.

2. METHODS USED

2.1 Strain Methods

The strain methods use the horizontal strains at the bottom of the AC layer to calculate the fatigue life of the pavement system, using laboratory derived fatigue equations. For multiple axles, the damage is calculated from several critical strains individually and then summed. The difference between the two strain methods lies in the strain values that are input into the fatigue equations. In this paper, only the peak and peak-midway strain methods are considered.

Figure 1 shows typical longitudinal strain time histories under single and tandem axles. The peak method takes only the peak tension part of the strains (designated as ε_p in the figure) to calculate the fatigue life of the pavement system.

The peak-midway strain method accounts for both the peak tensile strain and the peak compressive strain of the longitudinal strain time histories. The difference of the peak tensile and compressive strains (designated as ε_{pm} in Figure 1) is input in empirical fatigue equations to calculate the fatigue life and the damage of the pavement. It should be noted that there is no fatigue testing done with this type of loading pulse.

Figure 2 shows typical transverse strain time histories under single and tandem axles. The peak method is theoretically identical to the peak-midway strain method for the transverse strain under a single axle. However, for transverse strains under multiple axles, this method neglects the interaction between the adjacent axles and treats them as two separate single axles. In other words, it considers the two peak strain values (ϵ_1 and ϵ_{p1} in Figure 2 (b)) separately such that it does not differentiate between the tandem axle and two separate single axles.

The peak-midway method, on the other hand, takes the peak tensile strain due to the first axle (shown as ε_1 in Figure 2 (b)) and the difference of the second peak and the valley in between (shown as ε_{pm1} in Figure 2 (b)). Thus, this method considers the interaction between the two axles of the tandem axle.

For the transverse strain time history under a single axle (figure 2 (a)) which only has tension, the peak strain value is input into the fatigue equation to calculate the N_f. For the longitudinal strain under a single (figure 1 (a)) which includes tension and compression, there are two possible inputs into the fatigue equation: Inputting either ε_p or ε_{pm} would result in two different LEF values.

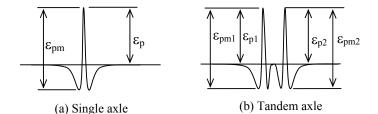


Figure 1. Typical longitudinal strain time histories

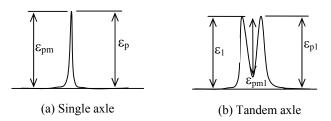


Figure 2. Typical transverse strain time histories

A better way to do this would be to use a loading pulse similar to what is observed (separately from for transverse and longitudinal directions) to develop the fatigue curves, and then use the corresponding strain values to calculate N_f . In practice, this has not been done. Instead, the transverse or longitudinal strains are frequently input into the fatigue equations that are based on pulse or sinusoidal loading without taking into account the above considerations.

2.2 Dissipated Energy Method

Dissipated energy is defined as the area within a stress-strain hysteresis loop. It represents the energy lost in the pavement due to the passage of an axle group. Figures 3 and 4 show the longitudinal and transverse stress-strain loops for single and tridem axles, respectively.

The advantage of this concept is that the dissipated energy can be calculated as a single scalar value and put into the fatigue equations to calculate the damage directly. This procedure eliminates the summation of damage due to several critical strain values that is necessary for the strain methods. Furthermore, the dissipated energy value captures the totality of the stress-strain response during the passage of the load(s) while the strain values correspond to only one point in time. The method also differentiates between multi axles and several independent single axles naturally.

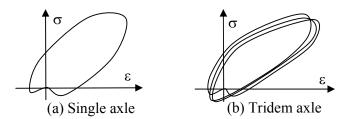


Figure 3. Longitudinal stress-strain hysteresis loop

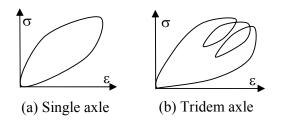


Figure 4. Transverse stress-strain hysteresis loop

However, similar to the discussion above on strains, if the dissipated energies due to a passage of an axle (single or multiple) in the longitudinal and transverse direction are input into the same fatigue equation they will result in different LEF values, even though both loops correspond to a given axle type.

3. FATIGUE EQUATIONS

Fatigue tests using the Indirect Tensile Cyclic Load Test (ITCLT) have been performed at Michigan State University (El Mohtar, 2003). The laboratory tests were based on the transverse loading pulses of several axle types. The interaction level between the axles is defined as the ratio of the peak strain to that of the valley. In this study, three levels of interaction were tested; 25% (low), 50% (medium), and 75% (high) as shown in figure 5. Figure 6 shows the test results based on dissipated energy. The results suggest the following equation regardless of axle type, loading mode, and interaction level:

$$N_f = 2.12 \times w_0^{-0.955}$$

where N_f is the fatigue life and w_0 is the initial dissipated energy density (in psi). The fact that the fatigue curve based on dissipated energy could be applied irrespective of axle type, loading mode and interaction level provide a great advantage to the dissipated energy approach relative to the strain based apporach. For multiple axles, the two different strain methods will give different N_f values whereas the dissipated energy method will give one unique N_f value. Thus, comparing the strain methods with the dissipated energy method will allow for determining which strain method works better for estimating the LEF values of multiple axles.

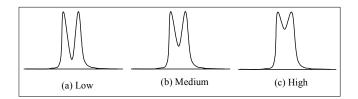


Figure 5. Different interaction levels

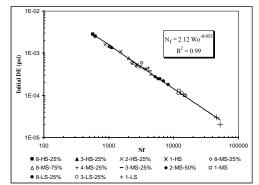


Figure 6. Laboratory results – N_f vs. initial dissipated energy

From the Laboratory data the strain-based fatigue equation was also developed. This equation is based on single pulse and transverse strain.

$$N_f = 5.97 \times 10^{-7} \times \varepsilon_0^{-2.342}$$

where N_f is the fatigue life and ε_0 is the initial strain.

Fatigue equations based on longitudinal strains or compression-tension loading could not be developed due to the limitations of the testing apparatus.

4. ANALYSIS

4.1 Generating Theoretical Stress-Strain Time Histories

The stress and strain time histories at the bottom of the asphalt layer were generated using the SAPSI-M computer program (Chatti and Yun, 1996). Figure 7 shows typical longitudinal and transverse strain time histories under a tandem axle. The details of shape and characteristics of the time histories are well described elsewhere (Chatti et al, 2000), and therefore are not included herein.

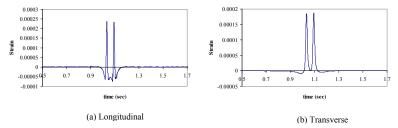


Figure 7. Longitudinal and transverse strain time histories generated by SAPSI-M

The pavement profiles and the axle configurations used in this paper are summarized in Tables 1 and 2 respectively. All axles presented here are composed of dual tires except for the front steering axle. Table 3 shows the trucks analyzed in this study.

The tire pressure was held constant at 689 kPa (100 ksi). As a result, the tire-pavement contact area was varied as the load of the axle varied. To calculate the LEFs, the fatigue life of a standard axle with dual tires and a tire pressure of 483 kPa (70 ksi) was also calculated.

Table 1. Pavement profiles used

Pavement Profile Type	AC Thickness, mm(in)	AC Modulus, MPa (ksi)	AC Damping Ratio			
Thin, Stiff, Low Damping	90 (3.5)	4830 (700)	0.05			
Thin, Stiff, High Damping	90 (3.5)	4830 (700)	0.10			
Medium, Stiff, Low Damping	203 (8)	4830 (700)	0.05			
Medium, Stiff, High Damping	203 (8)	4830 (700)	0.10			
Thick, Stiff, Low Damping	305 (12)	4830 (700)	0.05			
Thick, Stiff, High Damping	305 (12)	4830 (700)	0.10			

Table 2. Axle types and loads

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Axle type	Load per axle, kN (kips)			
Standard axle	80 (18)			
Front steering axle	69 (15.4)			
Single axle	80 (18)			
Tandem1 axle	71 (16)			
Tandem2 axle	58 (13)			
Tridem axle	58 (13)			
Quad axle	58 (13)			
5 axles	58 (13)			
7 axles	58 (13)			
8 axles	58 (13)			

Table 3. Trucks analyzed and their gross weights

Truck No.	Shape	Gross Weight kN (kips)	Truck No.	Shape	Gross Weight kN (kips)
1		149 (33.4)	6		674 (151.4)
2	Ţ Ţ	211 (47.4)	7		718 (161.4)
3		242 (54.4)	8		589 (132.4)
4		300 (67.4)	9		616 (138.4)
5	Ģl _æ , , , , , ,	531 (119.4)	10		674 (151.4)

4.2 Results and Discussions

As discussed above, the LEF is defined as the damage due to the passage of a given axle relative to a standard axle. That is, using the dissipated energy the LEF is calculated as:

$$LEF_{axle} = \frac{Damage_{axle}}{Damage_{s \tan dard}} = \frac{\frac{1}{N_{f,axle}}}{\frac{1}{N_{f,s \tan dard}}} = \frac{N_{f,s \tan dard}}{N_{f,axle}} = \frac{\alpha_1 \cdot w_{o,s \tan dard}}{\alpha_1 \cdot w_{o,axle}} = \left(\frac{w_{o,s \tan dard}}{w_{o,axle}}\right)^{\beta_1}$$

Similarly, using the strain equation the LEF is:

$$LEF_{axle} = \frac{Damage_{axle}}{Damage_{s \tan dard}} = \frac{\frac{1}{N_{f,axle}}}{\frac{1}{N_{f,s} \tan dard}} = \frac{N_{f,s \tan dard}}{N_{f,axle}} = \frac{\alpha_2 \cdot \varepsilon_{o,s \tan dard}}{\alpha_2 \cdot \varepsilon_{o,axle}} = \left(\frac{\varepsilon_{o,s \tan dard}}{\varepsilon_{o,axle}}\right)^{\beta_2}$$

These LEF values will be different with regard to which dissipated energy or strain values are used; longitudinal or transverse, as mentioned above. When a certain truck or a tire passes over a pavement system repeatedly the system will fail after a certain number of repetitions (N_f). At the same time, the passage of the load will create both longitudinal and transverse responses (stresses, strains and dissipated energy) that may not be the same in magnitude. If the longitudinal response

(strain/dissipated energy) value is input into a fatigue equation that was developed using the initial transverse response (strain/dissipated energy) values, the resulting N_f would not be the same. In addition, within a directional response (either longitudinal or transverse) the N_f value obtained from the initial strain should be the same as the one that was calculated from the corresponding dissipated energy. This logic also applies for the laboratory testing. Since the laboratory testing was based on only transverse loading, the fatigue equations are based on initial transverse strain and dissipated energy. However, one sample has only one N_f value but two response values: initial dissipated energy and initial strain. When those two response values are input into the corresponding fatigue equations, respectively, they should result in the same N_f value.

Nevertheless, when the above fatigue equations were used to calculate the N_f using transverse responses from SAPSI-M, the N_f values were different. In other words, the N_f value from the initial strain and that from the initial dissipated energy were not the same. To compare the LEF values from SAPSI-M and from the laboratory tests, the N_f from SAPSI-M had to be corrected such that the N_f from strain and dissipated energy is the same, under a single transverse pulse.

To overcome this problem, using the responses under a single axle loading, the damping ratio of the AC layer was determined by extrapolation (see figure 8) so that the N_f values from the initial strain and the dissipated energy are the same. The corresponding damping ratio was to be 3.7%.

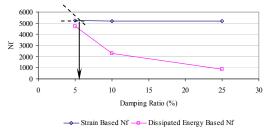


Figure 8. Extrapolation of damping ratio

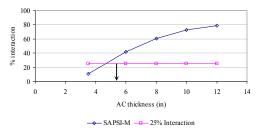


Figure 9. Interpolation of AC thickness (25% interaction)

In order to incorporate the interaction level for multiple axles, the thicknesses that resulted in 25%, 50%, and 75% interaction were determined by interpolation (see figure 9). The thicknesses were 122, 193, and 264 mm (4.9, 7.7, and 10.4 in.). The calculated LEF values (using the dissipated energy method) for these thicknesses with 3.7% damping ratio are shown in figure 10. Note that the LEF values shown in this figure are based on initial dissipated energy. As can be seen from the figure, the LEF from SAPSI-M are, in general, slightly higher than those from the laboratory testing, with the ratios of 1.19, 1.07, and 1.02 for 25%, 50% and 75% interaction, respectively. The following are observed from this figure:

- (1) Fatigue damage from multiple axles is significantly lower than single axles considering the load they carry
- (2) The higher the interaction level, the lower the fatigue damage.

Figure 11 shows the truck factors (TF) calculated for the trucks shown in table 3. The interaction level was kept constant at 25%. The TF values from SAPSI-M are higher than the laboratory results. This is expected since the LEF values from SAPSI-M are higher than the laboratory LEF values.

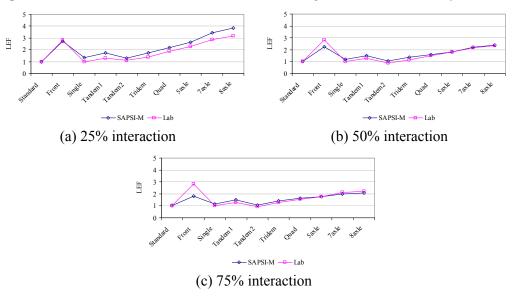


Figure 10. Comparison of LEF values from SAPSI-M and laboratory results

Note that, trucks 1 through 4 have truck factors that are close to 4.0. Because the maximum LEF of the rear axles of these trucks is approximately 2.0 (figure 10 (a)), it immediately implies that the transverse LEF of a front steering axle is significant. This will be discussed further below.

For multiple axles, different methods of estimating the LEF can be used. Figures 12 and 13 show the transverse LEF values calculated from three methods (dissipated energy, peak-midway strain and peak strain methods), using laboratory data and SAPSI-M results respectively. The peak strain method estimates the LEF values fairly well for single and tandem axles. However, for multiple axle groups that have 3 or more axles, the peak method starts to significantly overestimate the LEF values. This is because the peak method does not account for the interaction between the axles. Thus, the higher the interaction level, the more the peak strain method overestimates the LEF. On the other hand, the peak-midway strain method estimates the LEF value reasonably well as compared to the peak strain method, although it underestimates the fatigue damage relative to the dissipated energy method. Note that the peak-midway strain method takes the interaction level into account.

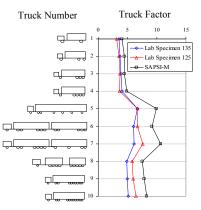


Figure 11. Truck Factors

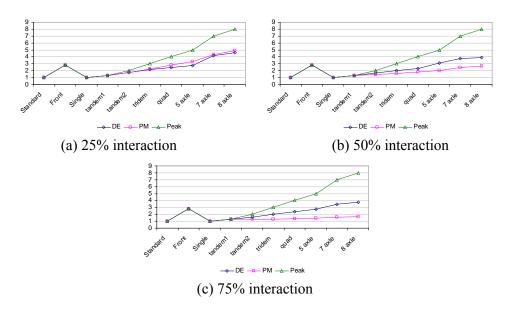


Figure 12. Comparison of transverse LEF values from different methods for 25% interaction (Laboratory)

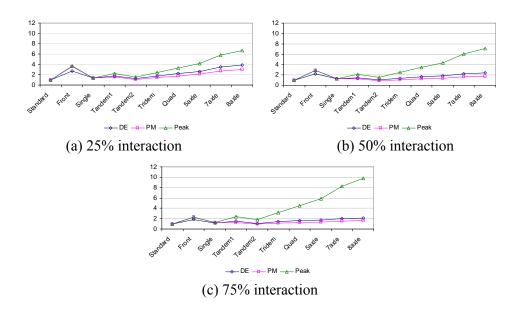


Figure 13. Comparison of transverse LEF values from different methods (SAPSI-M)

5. CONCLUSION

In this paper, the transverse LEF/TF values from the laboratory test results and the SAPSI-M computer program were compared. It showed that, the SAPSI-M based LEF/TF values agreed well with those from the laboratory results. For multiple axle groups, the peak-midway strain method agrees better with the dissipated energy method than the peak strain method. On the other hand, the peak strain method overestimates the transverse LEF values. This is because the peak strain method does not account for the interaction between the axles for the transverse response.

The results presented in this paper are based on fatigue equations developed using transverse pulse loadings. Longitudinal response based fatigue equations could not be developed due to testing

limitations. It is strongly recommended that similar analysis be conducted using fatigue equations developed from longitudinal pulse loadings.

REFERENCES

Chatti, K., D. Lee., T. Kim. 2000. Truck Damage Factors Using Dissipated Energy vs. Peak Strains. 6th International Symposium on Heavy Vehicle Weights and Dimensions. pp. 175-184.

Chatti, K. and K. Yun. 1996. SAPSI-M; Computer Program for Analyzing Asphalt Concrete Pavements Under Moving Arbitrary Loads. Transportation Research Record No. 1539, pp. 88-95.

El Mohtar, Chadi. 2003. Master's Thesis. Department of Civil & Environmental Engineering. Michigan State University. E. Lansing MI 48824.

Gillespie, T. D., S. M. Karamihas, M. W. Sayers, M. A. Nasim, W. Hansen, N. Ehsan, and D. Cebon. 1993. NCHRP Report 353; Effects of Heavy-Vehicle Characteristics on Pavement Response and Performance. TRB, National Research Council, Washington, D. C. 126 pp.

Hajek. J. J., and Agarwal. A. C. 2000. Influence of Axle Group Spacing on Pavement Damage. Transportation Research Record No. 1286, pp. 138-149.

Monismith, C. L. 1994. Fatigue Response of Asphalt-Aggregate Mixes. Strategic Highway Research Program, Project A-404, National Research Council.