# **RANDOM WAVES AND WAVE SPECTRUM**

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### SOME ACTUAL SAMPLE WAVE RECORDS



# **TWO WAYS TO ANALYZE RANDOM SEAS:**

A: Via Fourier Analysis of Wave Record  $\eta(t)$  at a location in the ocean:



 $\eta(t) \text{ can be expressed in terms of a series of sinusoidal waves with progressively decreasing periods: } T, \frac{T}{2}, \frac{T}{3}, \frac{T}{4}, \cdots, \frac{T}{J}, \cdots \text{ or progressively increasing (angular) frequencies:} \\ \omega = \frac{2\pi}{T}, 2\omega, 3\omega, 4\omega, \cdots, J\omega, \cdots \\ \eta(t) = \sum_{n=1}^{\infty} a_n \cos(n\omega t) + b_n \sin(n\omega t)$ (1)

 $a_n, b_n$ : Fourier coefficients ( $n^{th}$  harmonic)

or 
$$\eta(t) = \sum_{n=1}^{\infty} \frac{H_n^*}{2} \cos(n\omega t - \vartheta_n)$$
 (2)

 $H_n^*$ : height of  $n^{th}$  harmonic

 $\mathcal{G}_n$ : phase of  $n^{th}$  harmonic

$$H_n^* = 2\sqrt{a_n^2 + b_n^2}, \quad \tan \vartheta_n = \frac{b_n}{a_n}$$
(3)

**remember:**  $\cos(n\omega t - \vartheta_n) = \cos(n\omega t)\cos\vartheta_n + \sin(n\omega t)\sin\vartheta_n$ 

 $(H_n^*, \mathcal{G}_n)$  or  $(a_n, b_n)$  depend on the form of  $\eta(t)$ .

#### **Other notations:**

 $\omega_1 = \omega = \omega_0$ =fundamental frequency or  $1^{st}$  harmonic  $\omega_n = n \cdot \omega$ = multiple of fundamental frequency or  $n^{th}$  harmonic

The Fourier coefficients can be determined from the following integrals:

$$a_n = \frac{2}{T} \int_0^T \eta(t) \, \cos(n\omega t) dt \tag{4}$$

$$b_n = \frac{2}{T} \int_0^T \eta(t) \, \sin(n\omega t) dt \tag{5}$$

Note: In general there is a constant term  $[a_0]$  too, i.e.:

$$\eta(t) = a_{\circ} + \sum_{n=1}^{\infty} a_n \cos(n\omega t) + b_n \sin(n\omega t)$$
(6)

With 
$$a_{\circ} = \frac{1}{T} \int_{0}^{T} \eta(t) dt = \overline{\eta}$$
 = mean value of  $\eta$  over T (7)

In the case of wave profiles  $\eta$  is defined with respect to the SWL, thus:  $\overline{\eta} = a_0 = 0$ 

### Example:

### (you may also use the MULTICOMPONENT WAVE APPLET)

$$\eta(t) = \frac{H_1^*}{2} \cos(\omega_1 t - \vartheta_1) + \frac{H_2^*}{2} \cos(\omega_2 t - \vartheta_2) + \frac{H_4^*}{2} \cos(\omega_4 t - \vartheta_4)$$
(8)



It can be shown that the total energy under a given wave record is equal to the sum of the energies of its components (based on Parseval's theorem)

Remember: Specific Energy for each harmonic:  $\overline{E} = \rho g \frac{(H^*)^2}{8}$  or after dividing by  $\rho g$ we define:  $E^* = \frac{(H^*)^2}{2}$ Area =  $\frac{H_{10}^{*2}}{8}$  = Energy of the 10th harmonic  $m^2s$ Energy  $H_{10}^{*2}$  $\Delta E$ Spectral  $M_o = \text{area} = \int_{-\infty}^{\infty} S(\omega) d\omega$  = Total energy density  $8\omega_{\circ}$ Δω  $S(\omega) = \frac{dE^*}{d\omega}$ , S⁻¹ ω  $9\omega_0$  $10\omega_0$  $m^2$  $\Delta E^* = \frac{H_{10}^{*2}}{2}$ Cumulative specific energy  $M_o = \text{Total} \, E^* = \sum_n \frac{H_n^{*2}}{8}$  $E^* = \overline{E}/(\rho g)$  $\overline{\Delta}\omega = \omega_0$ ▶ S<sup>-1</sup> 5ω. 15ω 0  $20\omega_{\circ}$ -10ω<sub>-</sub>  $\omega = n\omega_0$  $9\widetilde{\omega_{0}}$ 

Total *wave energy* in a vertical column of water, under random waves, with sectional area A





### **B: Via Probabilistic Analysis of Wave Height Observations:**

Wave Height Distribution: Definition of Wave Height *Observation* (H) (zero-up-crossing method of Pierson, 1954)



**NOTE:** *H* should **NOT** be confused with  $H^*$ . Use the WAVE SPECTRUM APPLET to understand the differences.

 $\overline{P}(H) = \text{total number of wave heights between } H_1 \text{ and } H_2$   $N_{total} = \text{total number of wave heights}$   $\overline{P}(H) = PDF(H) = \frac{n}{N_{total}} \approx \overline{P}(\overline{H})(H_2 - H_1) \Rightarrow$   $\overline{P}(\overline{H}) = PDF(\overline{H}) \approx \frac{n}{N_{total}} \frac{1}{(H_2 - H_1)}$ 

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### Rayleigh Probability Density Function (PDF) $\overline{P}$ (H) (Longuet-Higgins, 1952)







**Cumulative Distribution Function:** P(H) =probability that  $H' \leq H$ 

Probability 
$$(0 < H' < H) = P(H) = \int_{0}^{H} \overline{P}(H') dH' = 1 - e^{-\left(\frac{H}{H_{rms}}\right)^{2}}$$
 (9)

Define: 
$$P^*(H) = (\text{probability that } H' \ge H) = 1 - P(H) \Rightarrow$$
  
 $P^*(H) = \text{Probability } (H' \ge H) = exc(\text{eedence})(H) = e^{-\left(\frac{H}{H_{rms}}\right)^2}$  (10)

Note that: Probability 
$$(H_1 \le H' \le H_2) = e^{-\left(\frac{H_1}{H_{rms}}\right)^2} - e^{-\left(\frac{H_2}{H_{rms}}\right)^2}$$

**Example:** If we wish to design a structure for a design height  $H_d$ , for which:

Prob (H>H<sub>d</sub>)=exc(H<sub>d</sub>)=1%=0.01 then:  $e^{-\left(\frac{H_d}{H_{rms}}\right)^2} = 0.01 \Rightarrow H_d = H_{rms}\sqrt{-\ln(0.01)} = 2.146H_{rms}$ This structure will exceed the design loading 0.01\*[24\*365\*3600 = # of secs in 1 year]/10=31,536 times (assuming an average period of 10 secs), and this information can be used to enter into a fatigue diagram to assess failure in fatigue.

# **Example**

The following wave heights were recorded in a fixed location in the ocean over a 12 hour period:

Range of H (m)	0-1.5	1.5-3.0	3.0-4.5	4.5-6.0	6.0-7.5	7.5-10.5
Number of wave	4000	6000	2000	500	300	60
height observations (n)						

Determine the PDF for the above waves. Find the  $H_{\rm rms}$  and compare the PDF with the Rayleigh PDF.

We need to plot  $\overline{P}(\overline{H})$  that corresponds to our samples. To do that we take as:

$$\overline{H} = \frac{H_1 + H_2}{2} \tag{11}$$

where  $H_1$  and  $H_2$  are the boundaries of each interval

Then: 
$$\overline{P}(\overline{H}) = \frac{n}{N_{total}} \frac{1}{H_2 - H_1}$$
 (12)

Because we want:  $\overline{P}(\overline{H})\Delta H = \frac{n}{N_{total}}$ 

where n=number of heights such that:  $H_1 < H < H_2$  and N=total number of heights

In our case:

$$N_{total} = 4,000 + 6,000 + 2,000 + 500 + 300 + 60 = 12,860$$

$H_1 \div H_2$	n/N <sub>total</sub>	$\Delta H = H_2 - H_1$	$\overline{H} = \frac{(H_1 + H_2)}{(H_1 + H_2)}$	$\overline{P}(\overline{H})$	$\overline{P}(\overline{H})$
			2	actual	Rayleigh
0÷1.5	0.311	1.5	0.75	0.207	0.192
$1.5 \div 3.0$	0.466	1.5	2.25	0.311	0.309
$\textbf{3.0} \div \textbf{4.5}$	0.155	1.5	3.75	0.104	0.148
4.5÷6.0	0.039	1.5	5.25	0.026	0.032
$6.0 \div 7.5$	0.023	1.5	6.75	0.015	0.0034
$\textbf{7.5} \div \textbf{10.5}$	0.005	3.0	9	0.0017	3.4×10 <sup>-5</sup>

$$H_{rms}^{2} = \sum \frac{n}{N_{total}} (H')^{2}$$
  
= 0.311×(0.75)<sup>2</sup> + 0.466×(2.25)<sup>2</sup> + 0.155×(3.75)<sup>2</sup> + 0.039×(5.25)<sup>2</sup> + 0.023×(6.75)<sup>2</sup> + 0.005×(9)<sup>2</sup> = 7.24m<sup>2</sup> → H\_{rms} = 2.69m

The Rayleigh PDF is then evaluated, using the following formula, with  $H_{rms}$ =2.69m and the values are shown on the Table above.

$$\overline{P}(\overline{H}) = \frac{2\overline{H}}{(H_{rms})^2} e^{-\left(\frac{\overline{H}}{H_{rms}}\right)^2}$$
(13)

### The Significant Height H<sub>s</sub>

### **Traditional Definition :** Average of the top 1/3 of wave heights

$$H_{s} = H_{\frac{1}{3}} = H_{33}$$



As  $\Delta H \rightarrow 0$ ,  $H_{\frac{1}{2}}$  becomes (note the denominator goes to 1/3, due to the definition of  $\hat{H}_{\frac{1}{2}}$ ):

$$H_{\frac{1}{3}} = \frac{\int_{\hat{H}_{1/3}}^{\infty} \overline{P}(H) H dH}{\int_{\hat{H}_{1/3}}^{\infty} \overline{P}(H) dH} = \text{centroid of } 1/3 \text{ top area of PDF}$$

and for Rayleigh PDF:

$$H_{s} = H_{\frac{1}{3}} = 1.416H_{rms} = \sqrt{2}H_{rms}$$

### **Example:**

a) Determine, in the case of Raleigh PDF, an expression for exc(H) in terms of  $H_s$ 

Probability 
$$(H' \ge H) = exc(H) = e^{-(H/H_{rms})^2} = e^{-2(H/H_s)^2}$$

Given that:  $H_s = \sqrt{2}H_{rms}$ 

### Note the above equation can be generalized as follows:

$$\exp(H) = e^{-a(H/Hs)^b}$$

Where a and b are appropriate parameters to match measurements.

a=2, b=2 for Rayleigh; a=2.28 & b=2.13 (Krogstad); a=2.26 & b=2.126 (Forristall)

### b) Determine *exc(H<sub>s</sub>)* for Rayleigh PDF:

Probability  $(H' \ge H_s) = exc(H_s) = e^{-2} = 0.135 = 13.5\%$ or, based on Forristall:  $exc(H_s) = e^{-2.26} = 0.104 = 10.4\%$ 

# Generalization of $H_{\frac{1}{3}}$



 $H_{n/N}$  = average of the highest n heights out of the total N heights.

$$H_{n/N} = \frac{\int_{\hat{H}_{n/N}}^{\infty} \overline{P}(H') H' dH'}{\int_{\hat{H}_{n/N}}^{\infty} \overline{P}(H') dH'}$$
(14)

where: 
$$\frac{n}{N} = e^{-\left(\frac{\hat{H}_{n/N}}{H_{rms}}\right)^2} \Rightarrow \frac{\hat{H}_{n/N}}{H_{rms}} = \left[-\ln\left(\frac{n}{N}\right)\right]^{\frac{1}{2}}$$
 (15)





### How well the Rayleigh PDF agrees with collected data? (Fig. 3-4 from SPM)



(the *exc(H)* is shown on the vertical axis)

Figure 3-4. Theoretical and observed wave height distributions. (Observed waves from 72 individual 15-minute observations from several Atlantic coast wave gages are superimposed on the Rayleigh distribution curve.)

### **Relationship between** $H_s$ with $M_0$

$$M_{0} = \int_{0}^{\infty} S(\omega) d\omega = \frac{Total \ Energy}{\rho g \times Area}$$
(16)

However: 
$$M_0 = \frac{1}{N} \sum_{n=1}^{N} \frac{H_n^2}{8}$$
 (17)

 $\equiv$  Average[energy/( $\rho g \times Area$ )]over all consecutive heights

From definition of  $H_{rms}$ :

$$H_{rms} \equiv \sqrt{\frac{1}{N} \sum_{n=1}^{N} H_n^2}$$
(18)

$$\Rightarrow M_0 = \frac{H_{rms}^2}{8} \to H_{rms} = 2\sqrt{2}\sqrt{M_0}$$
<sup>(19)</sup>

(remember:  $H_s = \sqrt{2}H_{rms}$  for Raleigh PDF)

$$H_{s} = \sqrt{2}H_{rms} = 4\sqrt{M_{0}} = 4\sqrt{\int_{0}^{\infty}S(\omega)d\omega}$$
<sup>(20)</sup>

The above definition of H<sub>s</sub> is more general and does NOT depend on the probability density function that the wave height observations might follow.

However, in the event the Rayleigh PDF applies, then the traditional definition of  $H_s = H_{1/3}$ , and the one above will define the same significant height.

The above is also shown in the next page:

# General definition of significant $H_s$





## The Significant Frequency and Period:



 $\omega_s$  = significant (or dominant) frequency (also called peak frequency  $\omega_p$ ) (corresponds to angular frequency of the wave harmonic with the largest amplitude)

$$H_s =$$
significant height =  $4\sqrt{M_0}$ 

### **Typical Wave Spectra:**



<u>NOTE</u>: Some wave spectra use  $f=\omega/2\pi$  instead of  $\omega$ . In that case:  $S(f)=2\pi S(\omega)$ , so that the area under S(f) represents the same amount of energy.

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 $\alpha = 8.1 \times$ 

 $\beta = 0.74$ 





$$S(\omega) = \frac{\alpha g^2}{\omega^5} \exp\left[-\beta (g/U\omega)^4\right]$$
(21)  
 $\alpha = 8.1 \times 10^{-3}$   
 $\beta = 0.74$   
 $g = 9.81m/s^2$   
 $U =$ wind speed in  $m/s$   
 $\omega =$ frequency in  $s^{-1}$   
It can be shown that:

$$M_{0} = \int_{0}^{\infty} S(\omega) d\omega = \frac{\alpha U^{4}}{4\beta g^{2}} (m^{2})$$
(22)

$$\frac{gH_s}{U^2} = 0.209$$
 Note:  $H_s \sim U^2$  (23)

$$\frac{gT_s}{U} = 7.16 \quad \text{Note: } T_s \sim U \tag{24}$$

### where $H_s$ and $T_s$ are the significant height and period, respectively.

### FDS Wave Spectra for various wind speeds

*Note Area under Spectrum* ~  $H_s^2 \sim U^4$ 



1 knot=1 nautical mile/hour = 1.151 mph = 0.514 m/s 1 nautical mile = 1,852 m 1 (statute) mile = 1,609.35 m

# Q: Can you explain why the peak of the wave spectrum moves to the "left" as the significant height (or the wind speed) increases?

Some other common wave spectra:





As we will see in the lecture on Wave Forces, an offshore structure must be analyzed subject to an incoming <u>mono-chromatic wave</u> of height  $H_{max}$  and period  $T_m$  corresponding to the <u>100-year storm</u>. We assume that this is the worst-case scenario.

# **Big Question:**

How can I determine  $H_{max}$  (height of 100-year storm) and  $T_m$  (period of the 100-year storm) for a certain location in the ocean?

# Answer:

If the location has been already known and developed (e.g. GOM, those parameters are given in API, as we will see on the section on Wave Forces). IMPORTANT NOTE: These parameters can be revised, based on information from recent STRONGER storms (e.g. *Ivan, Rita, Katrina, etc.*)

If it is a new location, then measurements, hindcasting can provide us with estimates for  $H_s$  (significant height) and  $T_p$  (peak period) over many past years. These collected data from the past years, theory (usually adjusted empirically), regression analysis, and extrapolations, can provide us estimates for  $H_{max}$  and  $T_m$ , as described in the lectures on Design Parameter Specifications in this course.

Problem 5 from the next list of problems and solutions presents an attempt to relate the maximum wave height to the significant wave height.

### Some Example Problems on Random Waves and Their Solutions:

3. A wave spectrum in *deep* water may be approximated by the shown triangle. Wave lengths larger than 80 m or less than 8 m are known to contribute *negligible* amounts of energy. It is known that the average value of the highest 4,000 out of 20,000 measured wave heights is equal to 1.3 m. (40 points)

a)

Determine the significant wave height and the peak value of the spectral energy density

b)

It is also known that the significant period of the wave spectrum is such that monochromatic waves with this period and wave height equal to *twice* the significant wave height, they

would be just about to break (in deep water). Determine the significant period of the waves

c)

Determine the percentage of the total energy contained in all waves with periods larger than the significant period



- 4. A sample of 15,000 consecutive wave height observations at a certain location in the ocean is considered. The average of the 1,000 highest of these waves has been found to be equal to 2.8 m. Determine the following: (25 points)
  - a)

The significant wave height

b)

The average of all the wave heights

c)

The *lowest* bound of the 20% highest waves

5. Prove that the maximum wave height,  $H_{max}$ , out of a sample on N wave height observations can be estimated by the following formula ( $H_s$  is the significant height of the sample): (20 points)

$$H_{max} = H_s (0.5 \ln N)^{0.5}$$

(2)

and their solutions:
Problem 3
Lmon = 80m , Lmin. = 8m
$L = \frac{gT^2}{2\pi} \longrightarrow T = \sqrt{\frac{2\pi L}{g}} \longrightarrow \omega = \frac{2\pi}{T} = \frac{2\pi}{\sqrt{\frac{2\pi L}{g}}} = \sqrt{\frac{2\pi g}{L^2}}$
$\omega_1 = \omega_{\min} = \sqrt{\frac{2\pi q}{80}} = 0.877 \text{ sec}^{-1}$
$\omega_2 = \omega_{\text{mon}} = \sqrt{\frac{2 \pi q}{8}} = 2.77 \text{ sec}^{-1}$
a) H <sub>1/5</sub> = 1.3 m = 1.56 Hzms (Fig 3-5, Theoretical server height distributions
$\Rightarrow$ H <sub>stms</sub> = 0.833 m
$H_{s} = \sqrt{2} H_{zm_{s}} \Longrightarrow H_{s} = 1.18 \text{ m}$
$5_{\text{max}} \times \frac{1}{2} \times (2.77 - 0.877) = M_0 = \frac{H_s^2}{16}$
$\implies$ $5_{max} = 0.0919 m^2 s$
b) $L_s = \frac{gT_s^2}{255}$ $\frac{2H_s}{L_s} = \frac{1}{7} \rightarrow L_s = 14H_s = 16.52m$
$\sum_{2H_s} \omega_s = \int_{L_s}^{2\pi g} = 1.93 \text{ sec}^{1}$
$T_{5} = \frac{2\pi}{\omega_{5}} = \frac{3.25}{3.25}$
C) $X = \frac{Area(ABD)}{Total(Area)} = \frac{BD}{BC} = \frac{\omega_s - \omega_1}{\omega_2 - \omega_1} = \frac{1.93 - 0.877}{2.77 - 0.877} = 0.556$
⇒ 55.6 %

$$\frac{Problem 4}{a}$$
a)  $H_{1/15} = 2 \cdot 8m = 1.9 H_{2ms} \rightarrow H_{2ms} = \frac{2 \cdot 8}{1 \cdot 9} = 1.473 \text{ m}$ 
 $H_5 = \sqrt{2} H_{2ms} = \frac{2 \cdot 0.8 \text{ m}}{1 \cdot 9}$ 
b)  $\overline{H} = 0.886 H_{2ms} = \frac{1.846 \text{ m}}{1 \cdot 9}$ 
c)  $P(H' > H) = \overline{e}^{(H/H_{2ms})^2}$ 
 $= 0.2$ 
 $-(\frac{H}{H_{2ms}})^2 = \ln(0.2)$ 
 $H = H_{2ms} \sqrt{-\ln(0.2)} = \frac{1.868 \text{ m}}{1 \cdot 9}$ 

Note in the case of Forristall, it can be shown that:  $H_{\text{max}} = H_s \left[ \frac{\ln N}{2.26} \right]^{0.47}$ 

For example if N=1,000, then  $H_{max}=1.86H_s$  from the former, and  $H_{max}=1.69H_s$ , from the latter formula.