

1 **TRANSPORTATION PROJECT OUTCOMES UNDER UNCERTAINTY:**
2 **AN EXAMINATION OF BENEFIT-COST RATIOS AND OTHER IMPACTS**

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26
27 **ABSTRACT**

28
29 Budget constraints and competing opportunities demand thoughtful project evaluation before
30 investment. Significant uncertainty surrounds travel choices, demographic futures, project costs,
31 and model parameters. The impacts of this uncertainty are explored by conducting hundreds of
32 sensitivity test runs across 28 random parameter sets to evaluate highway capacity expansion and
33 tolling project scenarios in Austin, Texas. The effects of different parameter sets on project
34 benefit-cost ratios, crash counts, emissions, traffic volumes, and tolling revenues are examined in
35 detail. Linear regression results show that link capacity, link-performance parameters – and their
36 covariation – are key to results, followed by the elasticity of demand, trip growth rates and
37 values of travel time.

38
39 **INTRODUCTION**

40
41 As a consequence of global recession, governments around the world are trimming budgets
42 (Economist 2011a, 2011b and 2011c). With U.S. gas taxes stagnant and transportation
43 construction prices rising 59% between 2000 and 2009 among six representative states (WSDOT
44 2011), transportation professionals must determine and pursue the most socially beneficial and
45 budget-sensitive projects possible, under tight funding constraints. Kockelman et al.'s (2010)
46 Project Evaluation Toolkit or PET allows users to quickly and with minimal input ascertain trip

47 tables for abstracted networks (Xie et al. 2010), anticipate demand shifts under different network
48 scenarios, and generate a host of project-evaluation metrics for side-by-side comparison. PET
49 anticipates emissions, crashes, traveler welfare, and network reliability impacts, relative to Base-
50 Case network conditions, and relies on user specification of project costs to estimate long-term
51 performance metrics (like internal rates of return, benefit-cost ratios, and net present values).

52
53 PET also enables sensitivity testing of project impacts, by allowing users to randomize 28 sets of
54 parameters (including values of travel time, link performance parameters, demand elasticities,
55 and regional growth rates, among others). Sensitivity testing allows basic project assumptions to
56 exhibit a degree of uncertainty and vary over the course of multiple trial runs, producing a
57 distribution for possible outcomes and giving analysts a sense of risks and rewards across project
58 alternatives.

59
60 An appreciation of the likely distributions of project outcomes is essential to wise decision
61 making, since actual outcomes can be much different than those expected. Standard & Poor's
62 Bain and Plantagie (2004) describe much of this problem, noting that estimates for some project
63 types have not only been inaccurate, but biased overall. In this light, transportation planners,
64 policymakers and investors may opt for a project with more certain benefits, rather than one with
65 slightly greater predicted benefits, but also a greater degree of uncertainty (with some outcomes
66 which may be particularly bad). Additionally, agencies may wish to package projects with a
67 significant degree of risk as a public-private partnership (Bain and Plantagie 2007).

68
69 This paper examines the potential impacts of upgrading an arterial street to a freeway or a
70 tollway in Austin, Texas. The nature of performance distributions is examined in greater detail
71 by varying 28 sets of parameters – one set at a time and in combination – in order to better
72 understand the impacts of multiple degrees of uncertainty across project scenario alternatives.
73 Simulation results suggest how much uncertainty exists in model predictions, with regression
74 results identifying key assumptions and inputs.

75 76 **BACKGROUND**

77
78 In 2003 Flyvbjerg et al. published an important cost-overrun study of 258 major public works
79 across 20 nations, emphasizing road projects (167 of the 258 cases) with the rest comprising rail,
80 bridges and tunnels. Two years later, a follow-up study (Flyvbjerg et al. 2005) focused on travel
81 demand forecasts, for 210 major rail and roadway projects. Both studies concluded that cost and
82 traffic estimates are highly uncertain, and regularly much different from actual values. For
83 example, road project costs averaged 20.4% higher than projected costs – with a standard
84 deviation of 29.9%, and half of all road projects had overstated demand by more than 20 percent,
85 with a quarter of estimates overstating demand by at least 40 percent. Rail-related biases were
86 even more dramatic, with 72% of all projects overstating ridership by 70 percent or more. Such
87 results highlight a substantial underlying degree of uncertainty in forecasting traffic flows (as
88 well as bias).

89
90 In their review of this literature, Lemp and Kockelman (2009b) noted that predicted traffic
91 volumes exceed actual volumes by over 30% in half of the hundreds of cases examined. Even
92 when correcting for optimism bias, uncertainty of traffic volumes and revenues remains

93 substantial, suggesting that analyst assumptions are far from perfect. To address at least the
94 variance in potential project outcomes, Lemp and Kockelman recommended that project
95 evaluations be conducted using Monte Carlo or related simulations to “provide probability
96 distributions of future traffic and revenue.” (2009b, p 1). This is consistent with the practice of
97 many others, including Ševčíková et al.’s (2007) model projecting future households by traffic
98 analysis zone, Gregor’s (2009) GreenSTEP emissions model which uses Monte Carlo sampling
99 to generate distributions and Wang’s (2008) application estimating uncertainty impacts using a
100 freight mode choice model. While such sampling addresses the variability underlying key
101 sources of project uncertainty, it does not address issues of model misspecification and bias.

102
103 The methods employed here use processes similar to those applied by Zhao and Kockelman
104 (2002), Pradhan and Kockelman (2002) and Krishnamurthy and Kockelman (2003) in their
105 investigations of uncertainty propagation through a standard four-step travel demand model and
106 integrated transport-land use models. For example, Krishnamurthy and Kockelman varied 95
107 parameters and two demographic inputs over 200 simulations. After generating model
108 predictions, they identified key inputs by regressing important outputs on the sets of variable
109 inputs. Results were most strongly impacted by changes in the link performance function
110 parameters, and shares of peak versus off-peak traffic (Krishnamurthy and Kockelman [2003]).
111 This investigation builds on these three previous works by looking at the value of specific
112 projects, rather than how urban-system model parameters affect total system travel distances and
113 land use patterns. PET produces a wide variety of parameter-dependent project-related impacts,
114 including emissions, crashes, traveler welfare, tolling revenues, and benefit-cost ratios.

115 116 **TOOLKIT DESCRIPTION**

117
118 The Project Evaluation Toolkit (PET) acts as a stand-alone tool for transportation project impact
119 assessment. PET is intended for use upstream of the NEPA process, allowing planners to
120 quickly evaluate a number of potential project variations before selecting the most appealing
121 candidate(s) for more detailed demand modeling (and more detailed networks). PET uses an
122 Excel interface with a C++-coded travel demand model that accommodates hundreds of network
123 links and relies on user-entered link volumes and attributes, plus hundreds of parameter values to
124 predict changes in travel patterns, emissions, crash counts, and other impacts (versus a base [e.g.,
125 no-build] case).

126
127 PET simultaneously serves several needs not currently met by any other single model. For
128 example, its data requirements and run times (less than an hour to evaluate three scenarios,
129 versus the Base-Case) are less cumbersome than required by regional planning models. PET
130 allows up to five traveler classes for assignment and infers trip tables from link counts, closely
131 mimicking traffic shifts on a complete network following network changes (Xie et al. 2010).
132 PET holistically evaluates full-network impacts, unlike other sketch-planning tools that lack
133 embedded travel demand models and focus on corridors. However, PET faces network-size
134 limitations (with run times growing exponentially with network links) and neglects land use
135 details (typically used for trip generation and attraction computations).

136
137 PET’s travel demand model operates by first estimating a base trip table from coded-link
138 volumes using a maximum entropy methodology (Xie et al. 2010). Next, PET performs an

139 iterative process to equilibrate travel times, costs, and flows, beginning with the application of
 140 elastic demand functions governing all origin-destination (O-D) pairs:

$$141$$

$$142 \quad x_{ij,d}^k = x_{ij,d}^{b,k} \left(\frac{g_{ij,d}^k}{g_{ij,d}^{b,k}} \right)^\eta \quad (1)$$

143 where $x_{ij,d}^k$ is the traffic volume of traveler class k (in vehicles per time period) between origin i
 144 and destination j during time-of-day period d , $g_{ij,d}^k$ represents the generalized cost (linearly
 145 combining time and money) of class k individuals traveling between origin i and destination j
 146 during time-of-day period d , and the superscript b denotes the Base-Case scenario traffic
 147 volumes or path costs. The η term represents the elasticity of trip-making and is set to -0.69
 148 based on weighted elasticities observed from application of regional travel demand models to the
 149 complete Austin network (Lemp and Kockelman 2009a). This function estimates changes in trip
 150 making for each user class, based on general travel cost changes between each O-D pair.

151
 152 After application of the elastic demand function, an incremental logit model (Ben-Akiva and
 153 Lerman 1985) is used to estimate changes in travel mode (e.g., SOV, HOV2, HOV3 or transit).
 154 For the heavy-truck driver class, the probability of choosing the heavy-truck mode is 1.0 so the
 155 mode split step is effectively ignored for these users. For other traveler classes, mode-split
 156 probabilities depend on user type (work-related [non-commute] travel, commuters and travelers
 157 with other non-work purposes), with each user type possessing distinct values of travel time and
 158 reliability. Their mode splits take the form:

$$159$$

$$160 \quad P_{ij,m}^k = \frac{P_{ij,m}^{b,k} e^{-\lambda_m \Delta g_{ij,m}^k}}{\sum_m P_{ij,m}^{b,k} e^{-\lambda_m \Delta g_{ij,m}^k}} \quad (2)$$

161
 162 In this model, $P_{ij,m}^k$ represents the probability that a traveler of type k originating at origin i and
 163 traveling to destination j will choose mode m ; and $\Delta g_{ij,m}^k$ represents changes in generalized
 164 travel costs (as defined earlier). The mode-choice model requires a single mode scale parameter
 165 (λ_m) to reflect the generalized cost term's coefficient in the associated systematic utility function
 166 (Ben-Akiva and Lerman 1985).
 167

168 The model then estimates changes in up to five time-of-day splits using a similar incremental
 169 logit model (with an associated time of day scale parameter, λ_t). Finally, the demand model
 170 relies on the Floyd-Warshall algorithm for shortest-path user equilibrium traffic assignment
 171 (Floyd 1962). This four-step iterative process (of elastic demand, mode and time-of-day choice,
 172 and network assignment, across multiple traveler classes) continues until equilibrium is reached
 173 using the method of successive averages. For more travel demand modeling details, see
 174 Kockelman et al. (2010).
 175

176 Once the demand model reaches convergence, traveler welfare impacts (consisting of changes in
 177 monetized travel times and operating costs plus any surplus from new travelers) are estimated for
 178 each O-D pair (ij), traveler class (k), and time of day (d) using the rule-of-half (RoH) (Geurs et
 179 al. 2010):
 180

$$\Delta CS_{ij,d}^k \cong \frac{1}{2} (w_{ij}^{b,k} x_{ij,d}^{b,k} + w_{ij}^k x_{ij,d}^k) (g_{ij,d}^{b,k} - g_{ij,d}^k) + w_{ij}^{b,k} x_{ij,d}^{b,k} (g_{ij,d}^{b,k} - g_{ij,d}^k) \quad (3)$$

where x 's represent each O-D pair's flow rate (before and after the network or policy change: x^b and x), g is generalized travel cost, and w is vehicle occupancy rate. This formulation accounts for benefits to new travelers who may be adding new trips due to reduced travel costs, as well as benefits to travelers who were already traveling from a given origin to a given destination, and see their travel costs fall. Preliminary testing was conducted based on Lemp and Kockelman's (2009a) demand model specifications to find that the RoH results very closely track (<5%) nested and standard logsums, provided that no major network changes are made or new alternatives are added (such as a subway mode).

All flows and welfare estimates are then imported to the Excel component, which estimates changes in network reliability, crashes, emissions, toll revenues, and fuel consumption. Reliability is estimated as a link-level travel time variance, using the following formula:

$$r_a = r_a^0 \left(1 + \sigma \left(\gamma + \frac{v_a}{c_a} \right)^\tau \right) \quad (4)$$

where r_a^0 is the free-flow travel time variance of link a , and σ , γ and τ are parameters estimated using traffic data obtained from freeway segments in Atlanta, Los Angeles, Seattle and Minneapolis (Margiotta, 2009). Using nonlinear least-squares regression, parameters were estimated to define the relationship between travel time variance and hourly volume-capacity ratios, with resulting values of $\sigma = 2.3$, $\gamma = 0.7$, and $\tau = 8.4$ (with an R^2_{adj} of 0.408) (Kockelman et al. 2010).

PET uses safety performance functions from Bonneson and Pratt's (2009) Roadway Safety Handbook to predict the total number of fatal plus injurious crashes on each directional link in the PET networks. Fatal and injurious count *shares* or splits, along with extrapolations of property damage only (PDO) crash counts, are then estimated from Texas crash data sets, (TxDOT 2009). Emissions estimates employ lookup tables generated using EPA's MOBILE 6.2, for 13 distinct species based on vehicle-fleet age and type distributions, ambient temperatures (summer and winter), model-estimated speeds, analysis year, and road facility type (freeway, arterial, ramp, etc.). Local calibration factors may be used to scale up or down crash counts and emissions volumes (due to local area crash histories, atmospheric variations, vehicle technologies, and so forth). For example, a 1.1 local crash calibration factor indicates 10% more crashes are expected than using default formulae. Summary measures are provided in the form of benefit-cost ratios, net present values, internal rates of return and payback periods (for each alternative policy or project, versus the Base-Case [no-build scenario]). PET's sensitivity testing module provides distributions on these, and many other model outputs, as illustrated in this work.

CASE STUDY

For this investigation, two scenarios were examined converting a four-lane arterial to a four-lane freeway or tollway. A 5% discount rate was assumed which is lower than the 7% required by the

226 OMB for federal projects, but is on the high end of the 3 to 5% discount rates typically used for
227 state transportation projects (FHWA 2007). Additionally, a 20-year design life was assumed
228 along with a 1% annual growth rate in Base-Case trip rates between all O-D pairs, though PET
229 has the ability to account for pair-specific growth rates. The 1% growth rate is lower than the
230 estimated regional population growth (Robinson 2008) but close to or higher than the expected
231 growth rate for zip codes in which the most congested roadways lie. Figure 1 illustrates the case
232 study location on the 194 link Austin regional network.
233

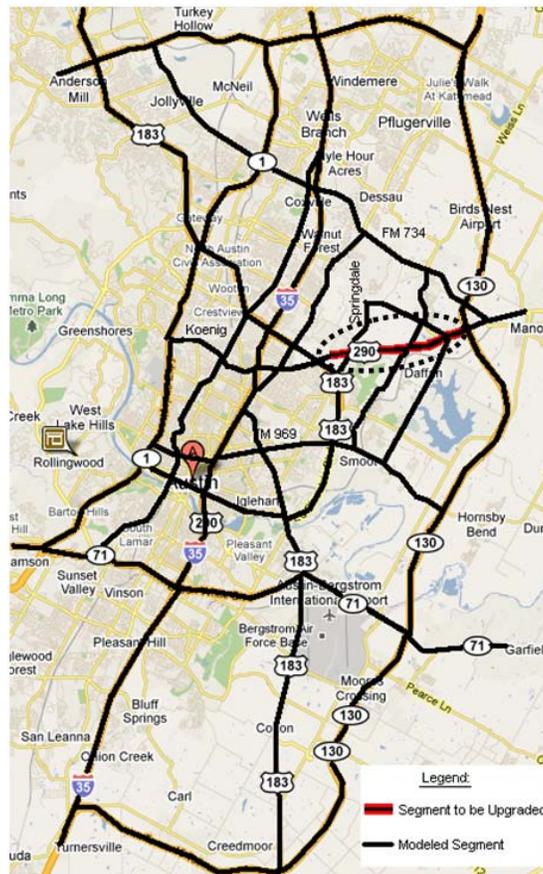


Figure 1: Case Study Location

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238 Both scenarios included upgrading the existing four-lane segment (two through lanes in each
239 direction) from an arterial to a grade-separated freeway or tollway, while retaining the two lanes
240 in each direction configuration. Two-way (total) capacity was estimated as increasing from 3080
241 vehicles per hour (vph) to 7640 vph, as well as eliminating seven intersections between US 290
242 and minor streets. The first scenario (Freeway) was modeled as a non-tolled freeway, and the
243 second scenario (Tollway) with fares at \$0.20 per mile for SOVs (similar to Austin's US 130
244 [TxDOT 2011]), \$0.10 per mile for HOVs (2 or more persons), no toll for transit users, and
245 \$0.60 per mile for heavy-trucks.
246

247 Initial project costs were estimated at \$71.8 million for the Freeway scenario and \$80.5 million
248 for the Tollway scenario, based on an estimated construction cost of \$3.2 million per freeway
249 lane-mile plus another \$760,000 per directional mile for installation of toll collection

250 infrastructure and 10 percent for design costs, as per recent Texas projects (TxDOT 2008). A
 251 \$30 million repaving project was also assumed needed 10 years after the initial-year in the Base-
 252 Case scenario. Annual maintenance and operations costs were estimated at \$410,000 for the
 253 Freeway scenario plus another \$1.13 million for the two tolling scenarios, based on recent Texas
 254 estimates (TxDOT 2008). Fagnant et al. (2011a) previously examined a similar case study, and
 255 changes in PET’s specification have resulted in somewhat different B/C ratios and other outputs.

256
 257 Due to rising input prices and the nature of this expansion project, the true tollway construction
 258 cost may be closer to \$7 million per lane-mile (as confirmed by Austin tollway expert Burford
 259 [2011]). A project recently was bid in the same location with a larger footprint (6 managed lanes
 260 + 6 frontage lanes) with a per-mile project bid cost indicating this new estimate (though costs
 261 could be lower than \$7 million per lane-mile due to the lack of new right-of-way acquisition).
 262 This variation of the base assumptions results in an approximate doubling of project costs and
 263 roughly a halving of these benefit-cost (B/C) ratios, as estimated below (parenthetically).

264
 265 Both alternative scenarios showed favorable B/C ratios. The Freeway scenario was most
 266 favorable from the public’s perspective, with a 14:1 B/C ratio, while the Tollway enjoys a
 267 respectable B/C ratio of 6.5:1. (These ratios are 7.7:1 and 3.5:1, respectively, under the higher
 268 construction cost assumption, of \$7 million per lane-mile.) The main reason for the Freeway
 269 alternative’s strong performance lies in its superior traveler welfare impacts, as shown in Table
 270 1:

271
 272 Table 1: Present Value of Capacity Expansion Scenario Impacts (in \$Millions)
 273

		Freeway	Tollway
Initial-Year	Total Impacts	\$32.0 M	\$12.4 M
	Traveler Welfare	\$23.8	\$5.0
	Reliability	\$7.0	\$6.3
	Crashes	\$0.7	\$0.7
	Emissions	\$0.5	\$0.4
Design-Year	Total Impacts	\$132.9 M	\$99.6 M
	Traveler Welfare	\$76.6	\$49.1
	Reliability	\$51.8	\$47.2
	Crashes	\$1.4	\$1.3
	Emissions	\$3.1	\$2.0

274
 275 In the Freeway scenario, travelers gain a mobility benefit without having to pay an extra fee, as
 276 in the tolling scenarios. However, this carries an implicit cost since the Freeway scenario must
 277 be financed through tax revenues. Conversely, the Tollway scenario is not only self-financing,
 278 but likely revenue generating with an estimated 23% internal rate of return (or 11% under the
 279 higher-construction-cost assumption) - to tolling authorities, rather than to society at large.

280
 281 While these estimates are high, and the projects may seem unusually attractive (from an
 282 engineering accounting standpoint), Austin tollway expert Burford (2011) feels that PET’s
 283 revenue projections appear reasonable. Transportation planners and policy makers may prefer

284 the Tollway scenario, since it offers a mechanism to quickly recover invested funds. This project
285 may be much more profitable than existing tollways in Austin, due to its lack of parallel (non-
286 tolled) frontage roads and assumption of no additional right-of-way requirements (which are
287 likely required, due to state laws that mandate provision of a “free” alternative to new tolled
288 routes).

289
290 Crashes and emissions also require further consideration, though outweighed by traveler welfare
291 and travel time reliability benefits when monetized. Over the 20-year design life, the projects are
292 projected to avoid 480-530 injurious crashes, 6 or 7 of which are expected to be fatal. Most
293 emissions types are forecasted to fall in the initial-year and all are lower in the design-year. In
294 particular network-wide emissions of hydrocarbons, butadiene, formaldehyde and acrolein all
295 fall by over 0.9% when comparing the Freeway scenario’s design-year with the Base-Case
296 scenario. This is particularly impressive when considering that the improved links handle only
297 1.45% of total system traffic. For both crashes and emissions, the Freeway scenario is preferred,
298 though the Tollway is still beneficial. The major reason for this is that some vehicles in the
299 Tollway scenario chose longer and slower routes along arterials, thus increasing emissions and
300 crash risks.

301
302 Average daily speeds on the upgraded segment increased in both scenarios relative to the Base-
303 Case scenario, showing a 23 mph (55 vs. 32 mph) difference in the initial-year and 31 mph (54
304 vs. 23 mph) by the design-year. US 290 Traffic volumes are predicted to increase in both
305 scenarios versus the Base-Case, with 160 and 275 vpd in the first year growing to 930 and 1100
306 by the design-year for the Tollway and Freeway scenarios, respectively.

307 **PARAMETER VARIATION**

308
309
310 Twenty-eight parameter sets were then varied during sensitivity analysis in order to determine
311 the impact of parameter variation on outcomes. All random draws originate from lognormal
312 distributions, where the corresponding/underlying normal random variable’s standard deviation
313 varies, as per user specification, and is centered at zero. These draws result in lognormal
314 distributions with means centered approximately at 1, with reported coefficients of variation
315 (CoV), where CoV equals the distribution’s standard deviation divided by the absolute value of
316 its mean. Variations were conducted by drawing an independent random value for each
317 parameter set. (For example, all user classes’ values of travel time have a single, shared draw for
318 a given iteration, so all move up or down together, to help ensure some necessary correlation.)
319 This random draw was then applied to the Base Case and all alternative scenarios for that
320 iteration, and to the initial and design-life years (with interpolation of project impacts in
321 intermediate years, to moderate computational burdens). Hundreds of iterations were run, for
322 hundreds of evaluations across all scenarios (each versus the corresponding Base Case scenario).

323
324 Two sets of runs were conducted with three hundred iterations each, the first run containing a
325 low degree of uncertainty (0.10 or 10.0% CoV for all parameter sets) and the second a higher
326 degree of uncertainty for most parameters (10.0% CoV for three parameter sets with a relatively
327 strong degree of certainty, 30.7% CoV for most parameters, and 53.3% CoV for four parameter
328 sets with a high degree of uncertainty). These lognormal CoVs correspond to draws from the
329 underlying (normal) random variables centered at 0 with standard deviations of 0.1, 0.3 and 0.5.

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Table 2 shows which parameters were varied and the CoV for each set of runs, as well as the default average parameter values. For a full listing of these and other default-parameter value sources, please see the PET Guidebook (Fagnant et al. 2011b).

Table 2: Sensitivity Testing Parameters and Assumed Variations

Parameter	Low CoV	High CoV	Mean Values Used
Value of Travel Time	10.0%	30.7%	\$5 to \$50 per hour
Value of Reliability	10.0%	30.7%	\$5 to \$50 per hour of travel-time std. dev.
Vehicle Operating Costs	10.0%	30.7%	\$0.20 to \$0.50 per mile
Crash Costs	10.0%	30.7%	\$7500 (PDO) to \$1.13M (Fatal)
Emissions Values	10.0%	53.3%	For 5 species, varies widely
Link Capacities	10.0%	10.0%	Varies based on indiv. hwy link
Link Performance Params. (α & β) for BPR Formula	10.0%	10.0%	Varies based on link class
Free-flow Speeds	10.0%	10.0%	Varies based on link class
Reliability Parameters (σ & τ)	10.0%	53.3%	2.3, 8.4
Local Crash Rate Calibration Factor	10.0%	30.7%	1.0
Emissions Rate Calibration Factor	10.0%	30.7%	1.0
Mode Scale Parameter	10.0%	53.3%	1.0
Time-of-day Scale Parameter	10.0%	53.3%	0.1
Ambient Temperatures	10.0%	30.7%	76 (April-Oct), 56 (Nov-March) degrees Fahrenheit
Average Vehicle Occupancies	10.0%	30.7%	Averages 1.6 across all modes
User Class Share: Heavy-Truck Driver (very high VOT)	10.0%	30.7%	5%
User Class Share: Work Related (high VOT)	10.0%	30.7%	10%
User Class Share: Commuter (high VOT)	10.0%	30.7%	20%
User Class Share: Non-Work Related (low VOT)	10.0%	30.7%	65%
Mode Probability: SOV	10.0%	30.7%	35.9%
Mode Probability: HOV2	10.0%	30.7%	33.3%
Mode Probability: HOV3	10.0%	30.7%	29.6%
Mode Probability: Transit	10.0%	30.7%	0.12%
Annual Trip Growth Rates (over time)	10.0%	30.7%	1% Annually
Demand Elasticity (for O-D pairs)	10.0%	30.7%	-0.69
Initial Project Costs	10.0%	30.7%	\$71.8M - \$80.5M
Maint. & Operat. Costs	10.0%	30.7%	\$409,000 - \$1.13M

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Note: User class shares and mode split shares must sum to one, so sets of drawn values were normalized (after mean-one draws were multiplied by base shares, and heavy-truck shares were removed from consideration).

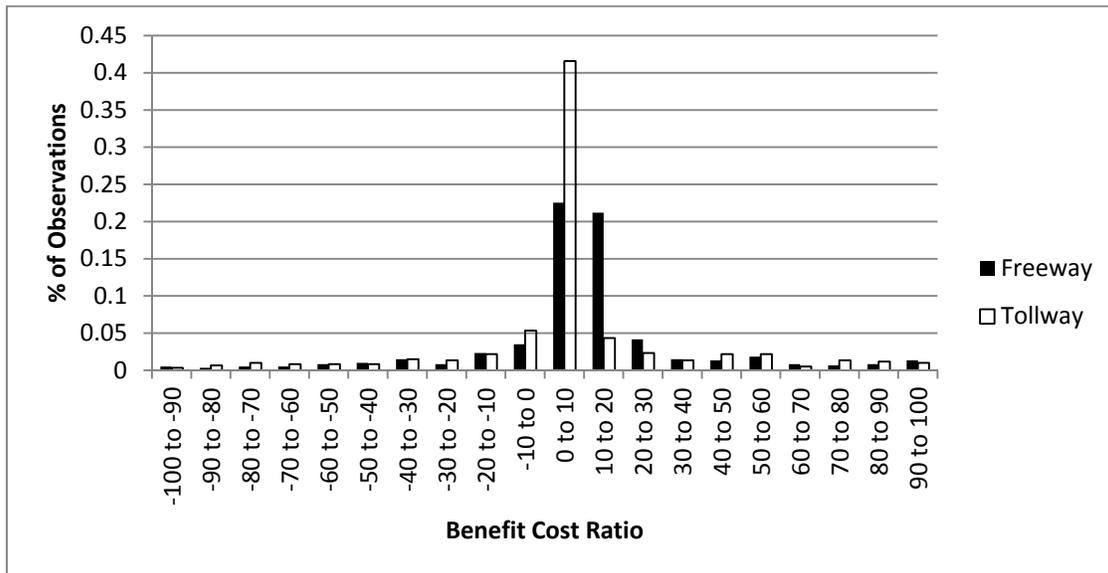
ANALYSIS OF SENSITIVITY TEST RESULTS

342
 343 Since benefit-cost (B/C) ratios drive many projects decisions, this output was examined first.
 344 B/C variations were dramatic, suggesting that input uncertainty can easily make or break a
 345 project. 15% of the 600 runs had B/C ratios below -100, and 17% had B/C ratios greater than
 346 100 in both scenarios. However, B/C output distribution of was very similar for both sets of
 347 sensitivity test parameters (i.e., both high and low CoV values), likely due to the invariance of
 348 CoV (held at 10 percent) in the BPR link-performance parameters (α , β , and link capacities, c).
 349 These sets of parameters are found to be key to impact assessment, since they regulate the
 350 estimated traffic speed on each traveled link (s_a) via the Bureau of Public Roads link
 351 performance function (TRB 2000), as follows:
 352

$$353 \quad s_a = \frac{s_a^0}{1 + \alpha \left(\frac{v_a}{c_a}\right)^\beta} \quad (5)$$

354
 355 where s_a^0 is the link's free-flow speed (obtained from Cambridge Systematics [2008]), v_a/c_a is
 356 the link's volume-capacity ratio, and α and β are behavioral parameters.
 357

358 Given their similar results, the low and high variation (CoV) sets of runs were combined for
 359 further evaluation. A histogram of the combined B/C ratios shows a very wide distribution of
 360 values, with a compact center, as shown in Figure 2.
 361



362
 363
 364 Figure 2: B/C Ratios (with values beyond +/- 100 not shown)
 365

366 Shares of B/C ratios were similar across both scenarios, with 54% (Freeway) and 56% (Tollway)
 367 of outcomes falling in the -20 to +30 band of reasonable B/C ratios, 21% (Freeway) and 19%
 368 (Tollway) falling below -20 and 25% (both scenarios) lying above +30. In other words, there
 369 was much more variation in performance measures across test runs than across project
 370 alternatives. Nevertheless, important differences across project alternatives can be observed near
 371 the median values. The median B/C value for the Freeway scenario was 10.3, compared to 4.0

372 for the Tollway scenario, as apparent in Figure 2's distribution spikes. In fact, in 63 percent of
373 alternative comparisons, the Freeway scenario bested its competitors (Base-Case and Tollway
374 scenarios) while the Tollway scenario was preferred in just 22 percent of trials. In the remaining
375 instances, both alternatives showed B/C ratios less than 1.0, indicating a Base-Case (no-build)
376 preference. This shows how complex transportation networks can have unpredictable
377 consequences (similar to Braess' Paradox), and how improving travel for some travelers (even at
378 zero cost) may negatively impact others, particularly when modeling elastic demand under
379 congested conditions.

380
381 Also of note, the *median* B/C ratios across both alternative scenarios were less than the B/C
382 ratios estimated at mean parameter values. When PET was run without parameter variation, the
383 scenarios yielded favorable B/C ratios of 14.1 and 6.2 for the Freeway and Tollway scenarios,
384 respectively. In both instances, the 14.1 and 6.2 values fell around the 62nd percentile of the
385 sensitivity-test outcomes, suggesting that false certainty in model parameter values can mask
386 potential project downsides.

387
388 One clear factor in extreme B/C cases is a dramatic increase (or decrease) in total VMT versus
389 the Base-Case scenario. In instances with B/C ratios lower than -100, VMT averaged a 24%
390 design-year decrease vs. the Base-Case scenario, compared to a 51% VMT increase in instances
391 where the V/C ratio was greater than 100 and an average VMT decrease of 1.7% for all other
392 instances. Initial-year comparisons show similar patterns, though to a much smaller degree
393 (1.4% average decrease vs. 4.8% average increase). Alternative scenarios' design-year VMTs
394 grew in almost all sampled runs, though sometimes at a lower rate than the corresponding Base-
395 Case scenario. Large VMT changes also coincided with dramatic changes in traveler welfare
396 and reliability. More VMT ties to higher welfare estimates for induced travelers (thanks to the
397 Rule of Half), but can congest roadways, resulting in negative reliability impacts and often
398 resulting in negative welfare impacts for existing travelers. Therefore, depending on the specific
399 nature of the VMT increase, it can quickly lead to either much higher or lower overall welfare
400 values.

401
402 Since each scenario is distinct (e.g., some are weak proposals and others strong), there is no
403 guarantee inputs will impact outputs similarly across scenarios. Therefore, regression analyses
404 were conducted separately for each scenario (using stepwise deletion and addition of input
405 values as covariates, with a p-value cutoff of 0.05). The best fits were found using the natural
406 log of the absolute value of the simulated B/C ratios. Other specifications were investigated,
407 using B/C ratios directly or attaching a sign to their logarithm (to reflect the original ratio's sign),
408 but these performed poorly (with R^2 values less than 0.11). This is likely due to extreme B/C
409 values or outliers (causing non-linear impacts for extreme outcomes) and common factors that
410 contributed to both positive and negative outliers. These regression results are shown for B/C
411 ratios in Table 3.

412
413 One important limitation of using $Y = \ln(|B/C|)$ is that it fails to predict whether a particular,
414 random setting will result in a win ($B/C > 1$) or a loss ($B/C < 1$). In the presence of extreme (and
415 unlikely) outcomes, it remains important to determine which input factors influence the direction
416 and sign of project impacts, in addition to magnitude. Beyond B/C values, crash counts,
417 emissions estimates, link volumes, toll revenues and other PET outputs exhibited similar

418 outlying values, with most outliers emerging in the design-year (rather than in the initial-year,
 419 which is unaffected by the trip growth rate factor). To address the issue of outcome sign,
 420 standard linear regressions were performed (using untransformed outputs – e.g., $Y = B/C$) on the
 421 middle 50% of initial-year values (in the B/C case) and middle 90% of initial-year outcomes (for
 422 other outputs), by simply discarding the top and bottom 25 or 5% of points, in order to eliminate
 423 the disproportionate impact of outliers. Such results are also shown, for the B/C values, in the
 424 final columns of Table 3, and in Tables 4 and 5 for other impacts.
 425

426 Table 3: B/C Ratios Regression Model Estimates for Freeway and Tollway Scenarios
 427

	$y = \ln(B/C \text{ Ratio})$		$y = B/C$ (50 % truncated sample)	
	Freeway	Tollway	Freeway	Tollway
Constant	2.408	1.879	38.522	20.889
Value of Travel Time	2.881	2.552	7.390	8.408
Value of Reliability	0.665			
Vehicle Operating Costs	-1.436	-0.938	-5.737	
Emissions Values			1.904	1.294
Link Capacities	-14.946	-17.950	-31.156	-39.275
Link Performance Params.	7.914	10.130	10.508	18.561
Free Flow Speeds				-7.278
Reliability Parameters			3.749	1.620
Emissions Rate Calibration Factor	-0.596			
Time of Day Scale Parameter	0.435	0.886	-1.599	
User Class Share: Heavy-Truck Driver (High VOT)	2.346	2.089	5.198	
User Class Share: Work Related (High VOT)				2.487
User Class Share: Non-Work Related (Low VOT)	-1.311	-1.020	-3.739	
Mode Probability: SOV		-0.576		
Mode Probability: HOV2	0.761			
Mode Probability: Transit			-3.338	-3.169
Annual Trip Growth Rate	3.207	3.853		5.994
Demand Elasticity	2.741	3.137		
Initial Project Costs	-1.306	-1.027	-8.351	-1.752
N_{obs}	600	600	300	300
R^2	0.655	0.732	0.403	0.438
R^2_{Adj}	0.647	0.727	0.380	0.419

428 Several significant findings emerge from Table 3’s parameter estimates. First, the signs on
 429 estimated parameters are the same using transformed and untransformed B/C values, in the two
 430 datasets ($n=600$ vs. $n=300$). Similarly, the most important factors in the first model are also key
 431 in the second. The results suggest that, while networks that congest more quickly (due to link-
 432

433 performance parameter value shifts), lower initial costs, and higher values of travel time, trip
434 growth and demand elasticity tend to produce more extreme B/C values, most lead to positive
435 B/C results, on average.

436
437 Such results are mostly intuitive, and encouraging. In less extreme input-set cases, α and β
438 increases and constraints on system capacity appear to benefit travelers greatly. Capacity
439 reductions make travel speeds more responsive to demand levels, thus enhancing the value of the
440 two scenarios' capacity increases. The importance of these parameters is consistent with
441 Krishnamurthy and Kockelman's (2003) propagation of uncertainty tests (in land use-
442 transportation model applications for Austin). Additionally, when the outcome results in a high
443 negative cost, it makes sense that further constriction of system capacity and increases in α and β
444 can make a bad situation worse. In the most extreme cases, low capacity and high α and β
445 values resulted in instances where system VMT was nearly 8 times larger or smaller in the two
446 expanded-capacity scenarios than in the same-iteration Base-Case scenario, thereby generating
447 the unlikely results.

448
449 As noted earlier, the importance of these parameters also explains why the B/C distributions for
450 the high- and low-variation (Table 2) sets of runs were so similar. Capacity values and the other
451 two link performance parameters (α and β) were modeled with a single 10% CoV in both sets of
452 sensitivity testing runs. While other parameters were allowed to vary more (in the high-variation
453 runs), capacity and link performance parameters remain the driving force behind B/C outcomes.
454 They clearly dominate results, suggesting that link-performance assumptions deserve careful
455 generation and treatment.

456
457 Though their parameter values are not quite as large, sizable increases in VOTTs and the share of
458 heavy-trucks (which effectively diminishes link capacities) also improved B/C ratios (Table 3).
459 Interestingly, the values of traffic growth and demand elasticity appear to have greater impact on
460 the size – rather than sign – of the B/C outcomes. All parameters with Table 3 coefficients
461 exceeding the project-cost coefficient are practically most important. Initial project costs
462 comprise 90% or more of these two scenario's project lifecycle costs and so serve as a useful
463 reference point: essentially, a doubling of initial costs should reduce the B/C ratio's magnitude
464 by about 50 percent.

465
466 The impact of parameter variation on other key impacts was also evaluated. These assessed
467 impacts included the impact of variation changes on crashes, emissions, traffic volumes on the
468 impacted segment and system-wide tolling revenues. Even with a benefit-cost ratio in hand,
469 each of these key measures is likely still independently important to decision makers attempting
470 to discern which alternative scenario to fund, if either. Crashes in this evaluation were
471 monetized, using crash valuations as noted by Blincoe et al. (2002) inflated to current (2010)
472 values. However, non-economic "soft" crash components (such as the value of life and pain and
473 suffering) were not monetized and should therefore be independently evaluated. Changes five
474 emissions species (Hydrocarbons, Nitrous Oxide, Carbon Monoxide, Particulate Matter < 2.5 μm
475 and Particulate Matter < 10 μm) were also monetized using EU data (Mailbach et al. 2008).
476 These emissions values may be important to cities seeking to meet air quality attainment goals
477 and the "monetary emissions benefits" output provides a framework for users measure broad

478 impacts across all five monetized species. Table 4 details the initial-year regression outputs for
 479 number of crashes and emissions costs:

481 Table 4: Impacts on Initial-Year Crashes & Emissions Costs (mid 90%)
 482

	Initial-Year Crashes (Fatal & Injurious)		Initial-Year Emissions (\$M)	
	Freeway	Tollway	Freeway	Tollway
Constant	-5.63	-7.23	-4.99	-3.93
Value of Travel Time	-1.94	-3.63		-0.25
Vehicle Operating Costs			0.20	
Emissions Values			-0.40	-0.39
Link Capacities	10.97	11.92	4.90	4.59
Link Performance Params.	-4.94	-4.83	-1.30	-1.15
<i>Free Flow Speeds</i>			<i>1.35</i>	<i>1.11</i>
Local Crash Calibration Factor	-16.86	-15.11		
Emissions Rate Calibration Factor			-0.54	-0.39
User Class Share: Heavy- Truck Driver (High VOT)		-2.01	-0.23	-0.24
User Class Share: Non- Work Related (Low VOT)	1.82	1.74		
Mode Probability: HOV 3+		1.91	0.22	
N_{obs}	540	540	540	540
R^2	0.563	0.479	0.522	0.451
R^2_{Adj}	0.558	0.473	0.515	0.443

483 Several fundamental inferences may be made from Table 4. As with the B/C ratio results,
 484 capacity and the link performance parameters were influential in predicting crash and emissions
 485 changes vs. the Base-Case scenario. As link capacities increase, more people travel in the
 486 alternative scenarios than the Base-Case, resulting in more crashes and lower emissions savings
 487 (or higher costs). One major difference between the crash and emissions models, however, is
 488 that the local crash calibration factor is more influential in predicting crashes than the emissions
 489 rate calibration factor for predicting emissions costs. This indicates that estimated crash
 490 predictions are much more stable than emissions costs, since a 10% increase in either value
 491 should result in a 10% respective increase in crashes or emissions.
 492

493 In addition to reviewing crash and emissions impacts, traffic volumes and tolling revenues were
 494 also analyzed. Transportation agencies spending money to improve a facility want to know how
 495 much it will be used. Furthermore, tolling revenues were not included in the overall benefit-cost
 496 ratio as collected tolls were assumed to be a benefit-neutral transfer payment from individual
 497 travelers to society (Kockelman et al. 2010). However, transportation agencies (or private
 498 enterprise) using PET will undoubtedly be interested in revenues as this may be the mechanism
 499 used to pay for the project. Additionally, PET's tolling output is structured such that system
 500

501 revenues are reported rather than just for the improved link. This is particularly key in this case
 502 study as the project could impact collected tolls on an adjacent priced facility.

503
 504 As with crashes and emissions, linear regression models were run for initial-year upgraded
 505 segment traffic volumes in both scenarios, though only for the changes in tolling revenues for the
 506 Tollway scenario, as shown in Table 5:

507
 508 Table 5: Impacts on Traffic Volume & Tolling Revenues (mid 90%)
 509

	Initial-Year US 290 AADT		Initial-Year Tolls (\$M)
	Freeway	Tollway	Tollway
Constant	1039	716	8.06
<i>Value of Travel Time</i>	72	1118	1.43
<i>Vehicle Operating Costs</i>	-51	1638	2.03
<i>Link Capacities</i>	-1591	-2488	-1.44
<i>Link Performance Params.</i>	722	662	
<i>Free Flow Speeds</i>		-1396	
Mode Scale Parameter	23	-195	0.56
Time of Day Scale Parameter			0.40
Average Vehicle Occupancy	56		0.60
<i>User Class Share: Heavy- Truck Driver (High VOT)</i>	84	246	2.93
User Class Share: Work Related (High VOT)	32		
User Class Share: Commuter (Mid VOTT)	-34		
<i>User Class Share: Non- Work Related (Low VOT)</i>	-62	-505	-0.71
Mode Probability: SOV		-253	0.76
Mode Probability: HOV 2	44		
Mode Probability: Transit		196	0.76
Demand Elasticity	-34		
N_{obs}	540	540	540
R^2	0.811	0.540	0.284
R^2_{Adj}	0.806	0.531	0.271

510
 511 Again, capacity and the link performance parameters were found to be highly influential in
 512 estimating traffic volumes. As capacity increases, travelers have less incentive to use the
 513 improved link, as opposed to other routes. However, for the Tollway scenario, numerous other
 514 inputs had substantial impact. Values of travel time (VOTTs) and operating costs appear critical
 515 in determining traveler route choices and revenues. When either fall, travelers choose alternative
 516 non-tolled routes. Also, user shares substantially impacted revenues and traffic volumes on the
 517 Tollway. The Heavy-Truck Driver user class has the highest VOTT (and pays the highest fare),
 518 while the Non-Work Related user class has the lowest VOTT. Therefore, any increases in the

519 proportion of the Heavy-Truck Driver user class or decreases in the Non-Work Related user class
 520 resulted in more travelers on US 290 and more tolling revenues.

521
 522 Finally, a series of log-linear ordinary least squares regression estimates was conducted on the
 523 natural-log transformed ($\ln|y|$) values of design-year crashes, emissions, traffic volumes and
 524 tolling revenues, as shown in Table 6. As with the B/C ratio estimates, the best fits were found
 525 using this transformation, though other specifications were investigated. Again, attaching a sign
 526 outside the transformation resulted in weak fit statistics, likely due to non-linear impacts for the
 527 more extreme outcomes and common factors that contributed to both positive and negative
 528 outliers.

529
 530 Table 6: Estimating the Variation in Design-Year Impacts
 531

	Crashes	Emissions	US 290 AADT	Tolls
	Freeway/Tollway	Freeway/Tollway	Freeway/Tollway	Tollway
Constant	0.84	13.24	6.33	16.209
Value of Travel Time	0.74	1.49	0.65	
Vehicle Operating Costs	-0.72	-1.20	-0.78 (Tollway)	
Link Capacities	-7.34	-15.58	-9.33	-2.158
Link Performance Params.	4.95	9.82	5.20	0.821
Local Crash Calibration Factor	0.84			
Emissions Rate Calibration Factor		1.03		
Mode Scale Parameter				0.156
Time of Day Scale Parameter		0.53	0.26 (Tollway)	
Ambient Temperatures		-0.55		
Average Vehicle Occupancy	0.43 (Tollway)			0.297
User Class Share: Heavy-Truck Driver (High VOT)	1.08 (Freeway) 0.65 (Tollway)	1.68	0.85	
User Class Share: Work Related (High VOT)			0.49	
User Class Share: Non-Work Related (Low VOT)	-0.45	-1.01	-0.62 (Freeway)	
Mode Probability: HOV 2	0.33	0.90	0.30	
Mode Probability: HOV 3+			0.35	
Annual Trip Growth Rate	2.01	3.52	1.89	0.713
Demand Elasticity	2.36	2.82	1.44	0.86
N_{obs}	1200	1200	1200	600
R^2	0.511	0.640	0.511	0.192
R^2_{Adj}	0.506	0.636	0.506	0.183

532 Note: Table shows OLS regression results of the natural log of crash counts, emissions tons, AADT, and
 533 tolls revenues on variable inputs.
 534

535 As noted earlier, the purpose of this investigation was to determine which factors most influence
536 the design-year output levels. Unsurprisingly, capacity and the link performance parameters
537 dominated the outcomes' magnitude in all cases, with outputs rising as capacity becomes
538 constrained. The next parameter sets exhibiting important impacts are consistent across all
539 alternatives: annual trip growth rate and demand elasticity. In all four models, these two
540 parameters had a greater impact than any other, excluding capacity and the link performance
541 parameters. This makes sense, since the impacts of a trip growth rate will be compounded over
542 time and demand elasticity will regulate the additional number of trips that occur as travel costs
543 change, both crucial elements in the Freeway and Tollway scenarios.

544 545 **CONCLUSION**

546
547 This paper conducted a thorough investigation into the impacts of parameter uncertainty on
548 highway project outcomes. Twenty-eight parameter variations and their effects on benefit-cost
549 ratios, crashes, emissions, traffic volumes and tolling revenues were examined in detail. From
550 this evaluation it quickly became clear that if analysts underestimate capacity or overestimate
551 link performance parameters the benefit-cost ratio may quickly become unreasonable. Another
552 crucial finding showed that the median B/C ratio was significantly lower than the B/C ratio when
553 no variation was assumed. This is particularly important as these results would lead analysts to
554 expect lower probable benefits from these scenarios than the no-variation case would suggest.

555
556 Even when omitting extreme outcome variations, capacity and the link performance parameters
557 had greater impact on B/C ratios, crashes, emissions, traffic volumes, and tolling revenues than
558 any other examined inputs. B/C ratios were strongly depended on VOTTs in both scenarios,
559 though the value of time (and operating costs) impacted the actual use and collected revenues of
560 the improved facility for the Tollway much more heavily than in the Freeway scenario. Crashes
561 were found insensitive to congestion relative to emissions, though both were impacted. Finally,
562 the magnitude of final design-year outcomes was strongly influenced by travel growth rate and
563 demand elasticity parameters.

564
565 In summary, this paper illustrated potential applications of PET and provides detailed analysis
566 conducted of its outputs using sensitivity testing and input variation. Transportation planners
567 may employ similar methods, using the Toolkit to produce a range of likely outcomes, rather
568 than a single point estimate. This paper details which parameter variations tend to cause the
569 greatest variations in impacts under two scenarios, though ultimate results will depend on the
570 nature of any future project under consideration. These methods and findings should enhance
571 the ability of decision makers to allocate limited transportation funding resources while
572 providing the most beneficial outcomes for society at large.

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