

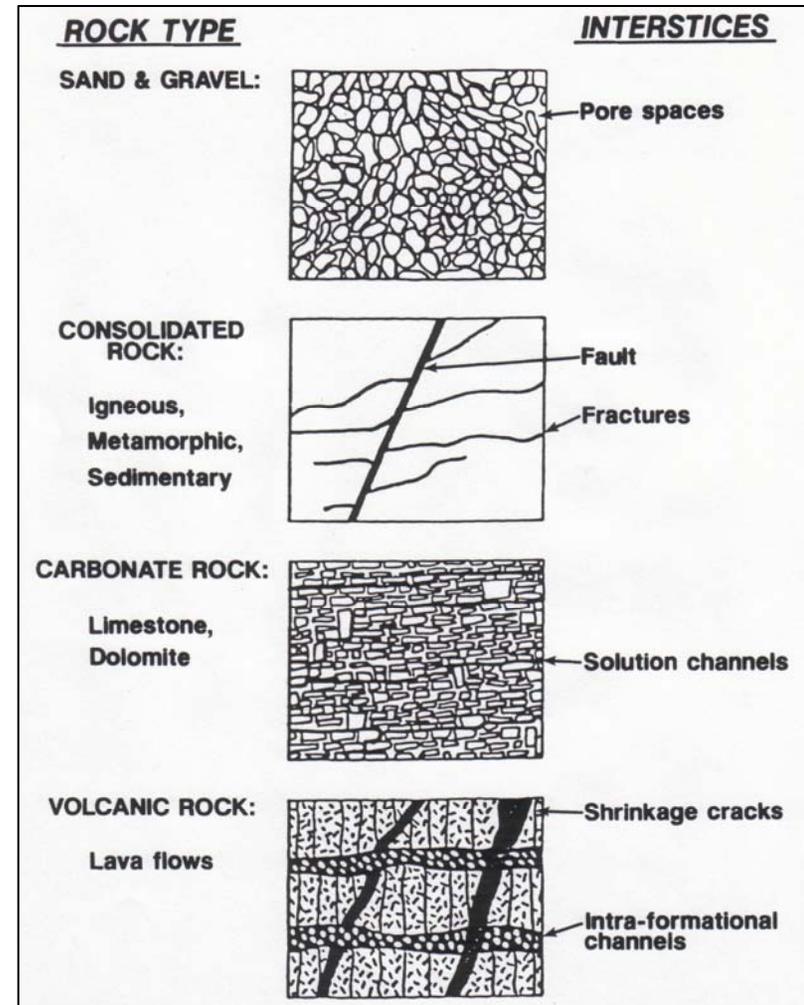
CE 374 K – Hydrology

Infiltration

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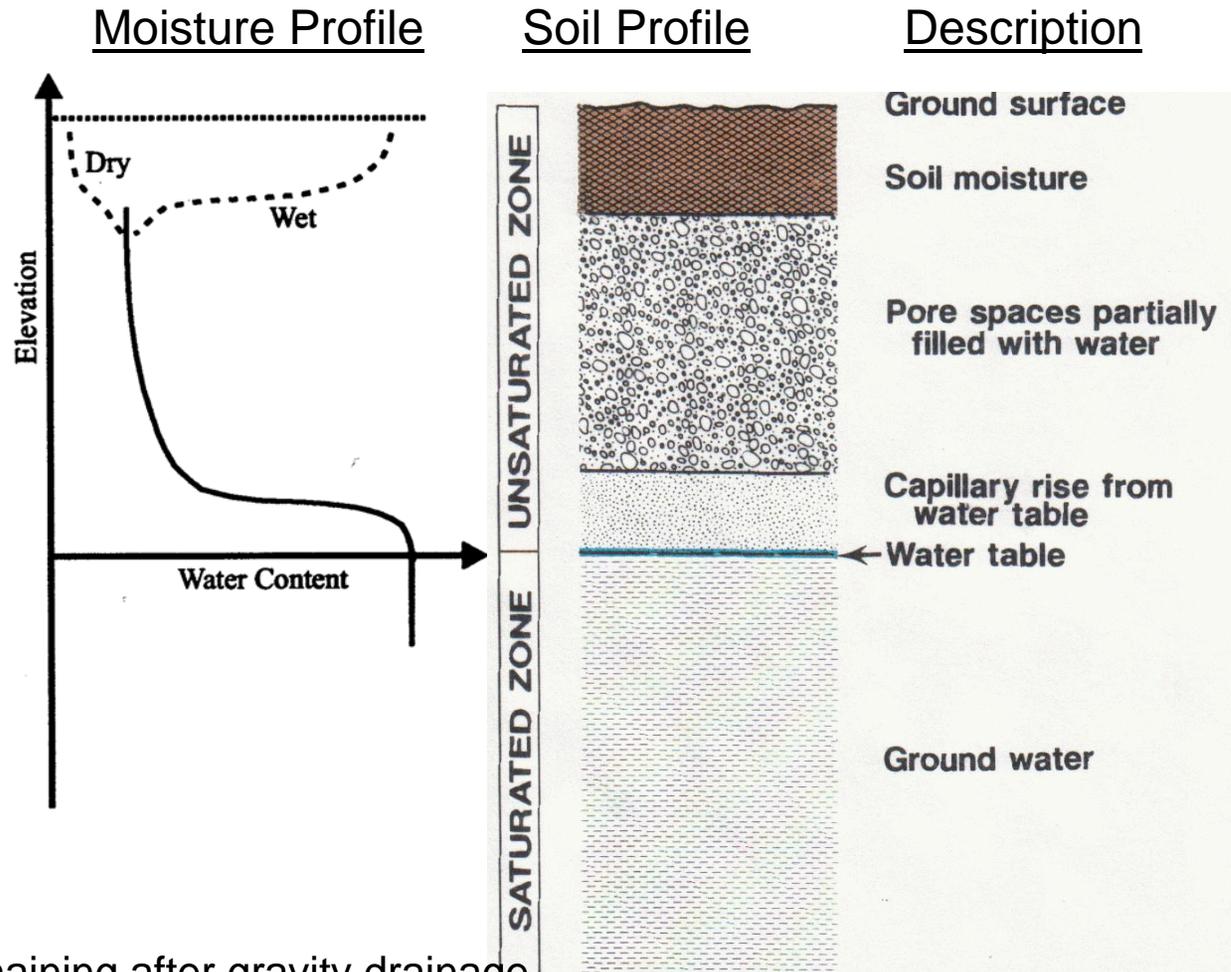
Porous Medium

- **Groundwater**
 - All waters found beneath the ground surface
 - Occupies pores (void space not occupied by solid matter)
- **Porous media**
 - Numerous pores of small size
 - Pores contain fluids (e.g., water and air)
 - Pores act as conduits for flow of fluids
- **Type of rocks in a formation and their**
 - Number, size, and arrangement of pores
 - Affect the storage and flow through a formation.
- **Pores shapes are irregular because**
 - differences in the minerals making up the rocks
 - geologic processes experienced by them.



Distribution of Subsurface Water

- **Different zones**
 - depend on % of pore space filled with water
- **Unsaturated Zone**
 - Water held by capillary forces, water content near field capacity except during infiltration
- **Soil zone**
 - Water moves down (up) during infiltration (evaporation)
- **Capillary fringe**
 - Saturated at base
 - Field capacity at top
- **Saturated Zone**
 - Fully saturated pores

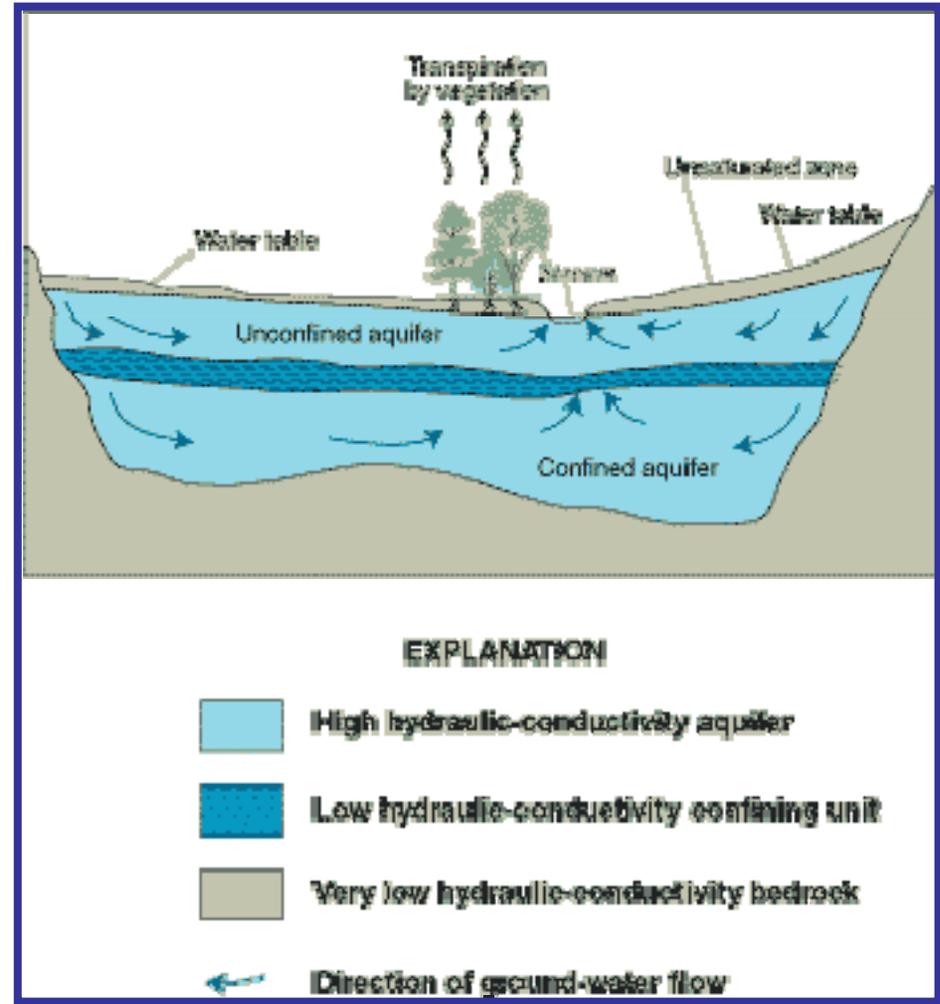


Field capacity - Water remaining after gravity drainage

Wilting point - Water remaining after gravity drainage & evapotranspiration

Aquifer Types

- **Aquifer** - store & transmit
 - Unconsolidated deposits sand and gravel, sandstones etc.
- **Aquiclude** – store, don't transmit
 - Clays and less shale
 - Impervious boundaries of aquifers
- **Aquitard** – transmit don't store
 - Shales and less clay
 - Leaky confining layers of aquifers
- **Confined aquifer**
 - Under pressure
 - Bounded by impervious layers
- **Unconfined aquifer**
 - Phreatic aquifer, water table aquifer
 - Bounded above by water table
 - Atmospheric pressure at water table



Porosity

Porosity – total volume that can be filled with water

Moisture content – actual volume filled with water

Saturation – fraction of pore volume filled with water

Porosity $\phi = \frac{V_i}{V}; \quad 0 \leq \phi \leq 1$

Moisture content $\theta = \frac{V_w}{V}; \quad 0 \leq \theta \leq n$

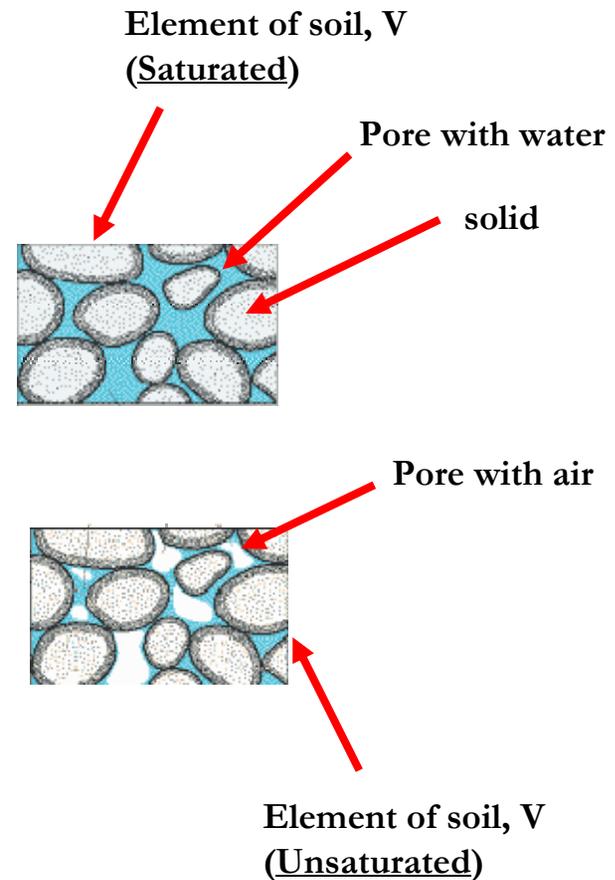
Saturation $S = \frac{V_w}{V_i} = \frac{\theta}{\phi}; \quad 0 \leq S \leq 1$

V = gross volume of element

V_i = volume of pores

V_s = volume of solids

V_w = volume of water



Aquifer Properties

- **Porosity** (n)
 - Percent of total pore space occupied by voids
- **Hydraulic conductivity** (K)
 - Ability of a formation to transmit water
- **Storativity** (S)
 - Ability of a formation to store water

Sedimentary Material	Porosity (%)
Peat Soil	60-80
Soils	50-60
Clay	45-55
Silt	40-50
Med. to Coarse Sand	35-40
Uniform Sand	30-40
Fine to Med Sand	30-35
Gravel	30-40
Gravel and Sand	30-35
Sandstone	10-20
Shale	1-10
Limestone	1-10

Piezometric Head

- Pressure distribution

$$\frac{\partial p}{\partial z} = -\gamma$$

- Piezometric head - energy per unit weight of the fluid

$$h = \frac{p}{\gamma} + z$$

↖ Elevation head
 ↖ Pressure head

h = piezometric head

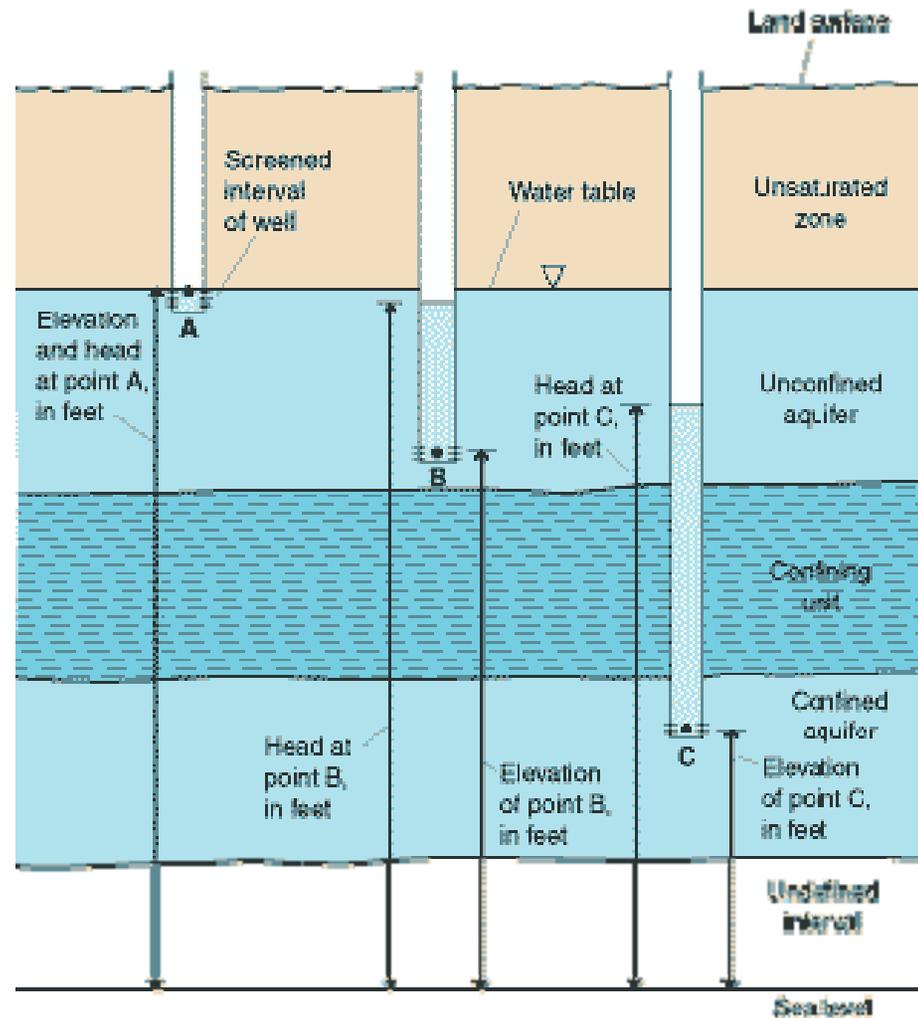
p = fluid pressure

$\gamma = \rho g$ = fluid specific weight

ρ = fluid density

g = gravitational constant

z = elevation





Dijon Fountain

Darcy's Law

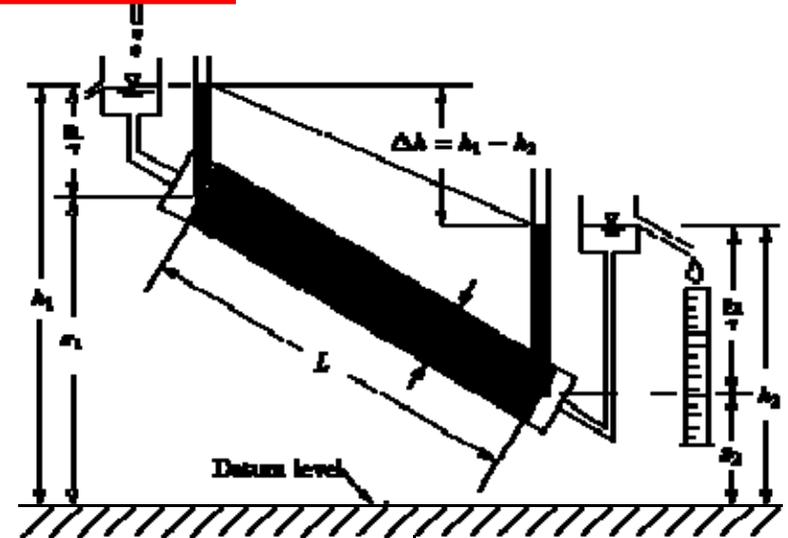
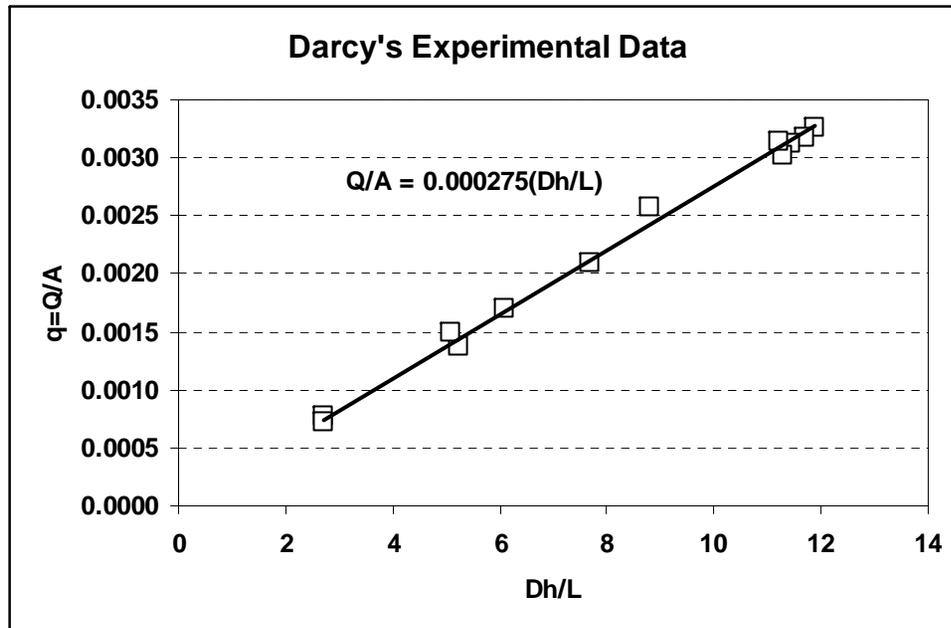
- Output is proportional to the head and inversely related to the length traversed



Henry Darcy

$$Q \propto A \frac{h_1 - h_2}{L}$$

$$q = \frac{Q}{A} = -K \frac{dh}{dz}$$



K = hydraulic conductivity
 $q = Q/A$ = specific discharge

Continuity Equation

$V = dx dy dz = \text{volume of element}$

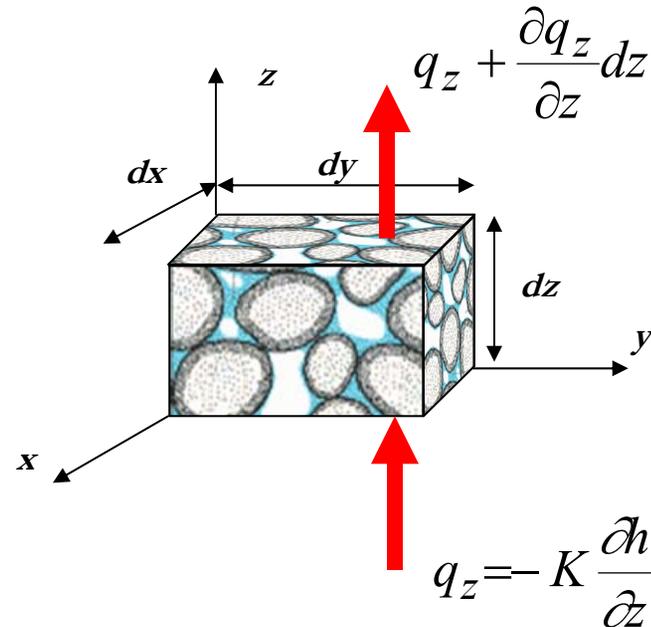
$V_w = \theta dx dy dz = \text{volume of water}$

$$\frac{d}{dt} \iiint_{CV} \rho_w dV + \iint_{CS} \rho_w \mathbf{V} \cdot d\mathbf{A} = 0$$

$$\rho_w dx dy dz \frac{d\theta}{dt} + \rho_w dz dx dy \frac{\partial q}{\partial z} = 0$$

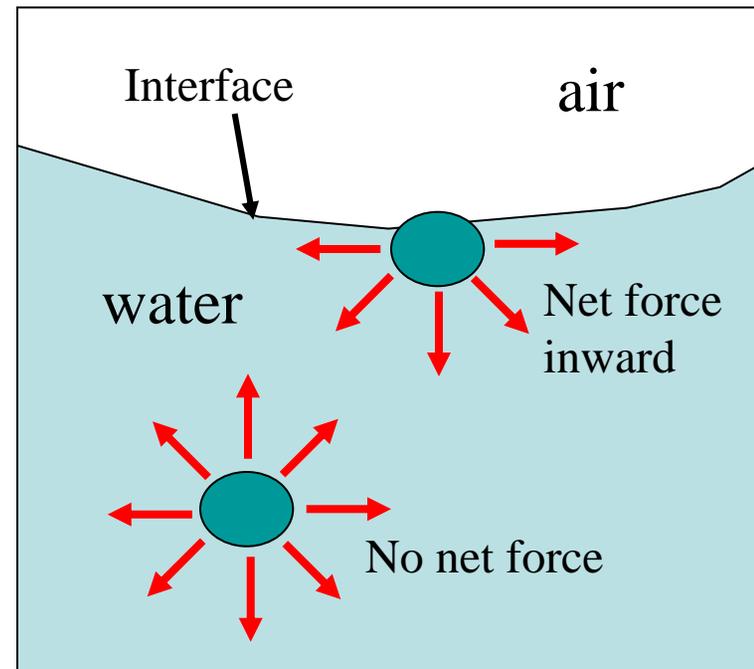
$$\frac{\partial \theta}{\partial t} + \frac{\partial q}{\partial z} = 0$$

Continuity
Equation



Surface Tension

- Below interface
 - forces act equally in all directions
- At interface
 - some forces are missing
 - pulls molecules down and together
 - like membrane exerting *tension* on the *surface*
- If interface is curved
 - higher pressure will exist on concave side
- Pressure increase
 - balanced by surface tension, σ
- $\sigma = 0.073 \text{ N/m}$ (@ 20°C)



Capillary Action

- Capillary Pressure
 - Related to surface tension and size of pores

$$P_c = \frac{2\sigma}{R} = P_{air} - P$$

$P_{air} = 0$

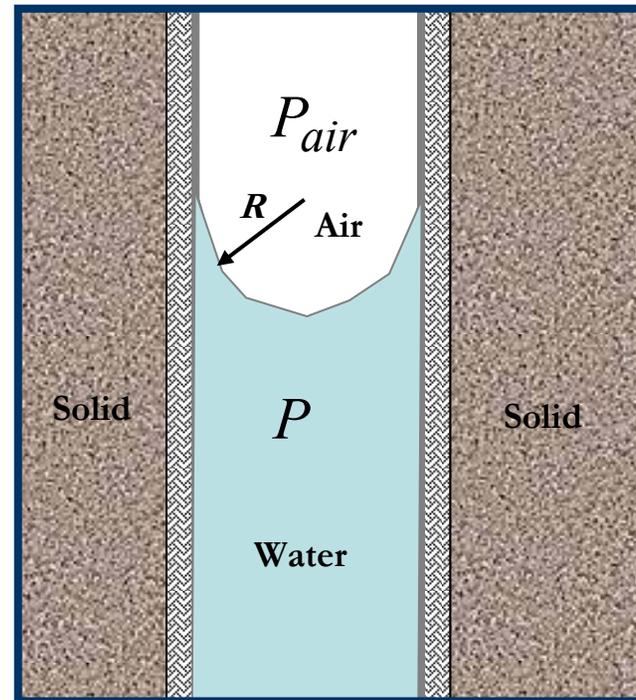
$$P_c = -P$$

- Related to water content

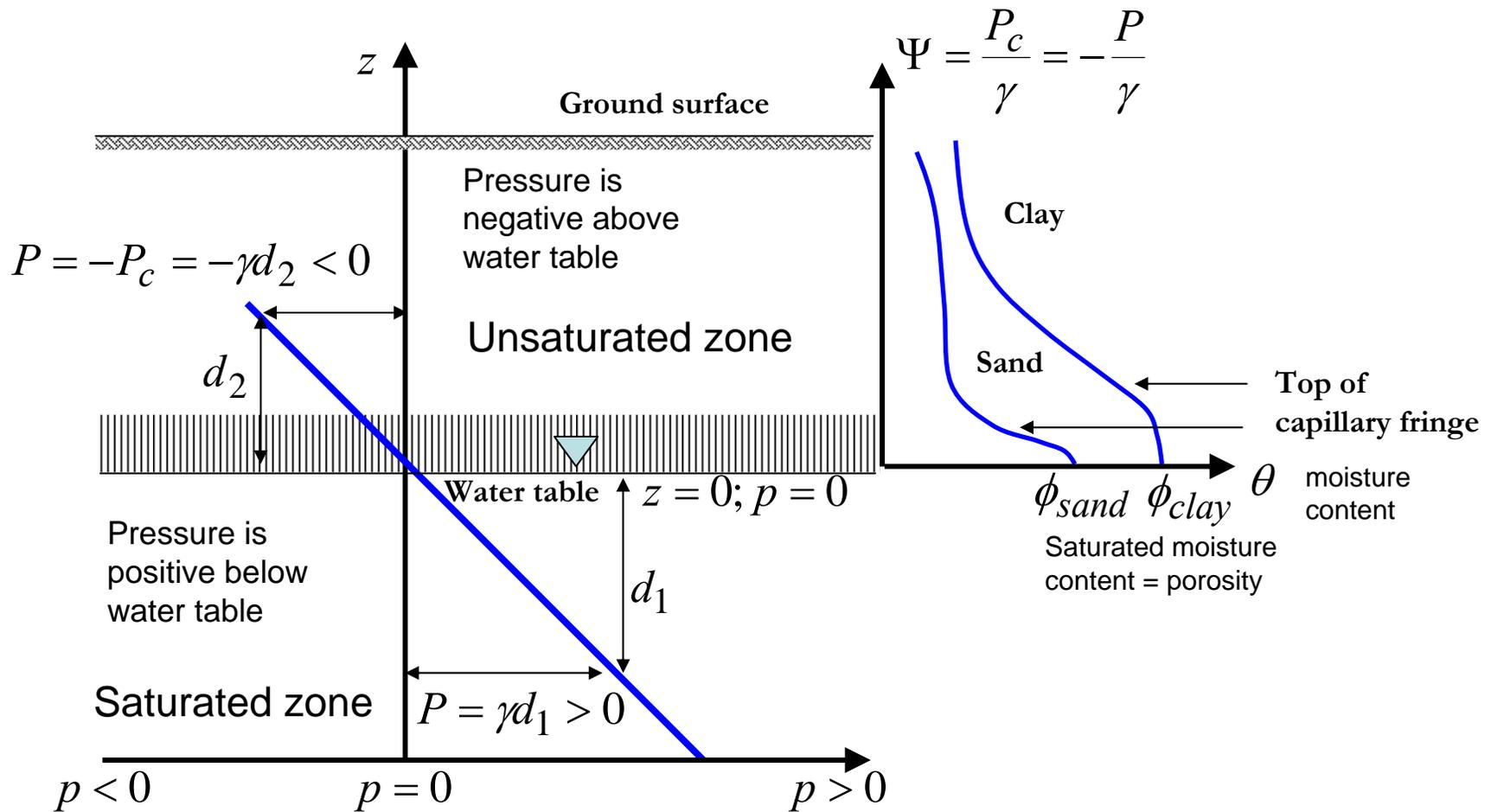
$$P_c = P_c(\theta)$$

- Suction Head $\Psi = \frac{P_c}{\gamma}$

- Total Head $h = \Psi + z$



Pressure Distribution in Subsurface



Richard's Equation

- Darcy's Law becomes

$$h = \Psi + z$$
$$q_z = -K \frac{\partial h}{\partial z} = -K \frac{\partial(\Psi + z)}{\partial z} = -\left(K \frac{\partial \Psi}{\partial \theta} \frac{\partial \theta}{\partial z} + K \right)$$
$$= -\left(D \frac{\partial \theta}{\partial z} + K \right)$$
$$D = K \frac{\partial \Psi}{\partial \theta}$$

Soil water diffusivity

- Continuity becomes

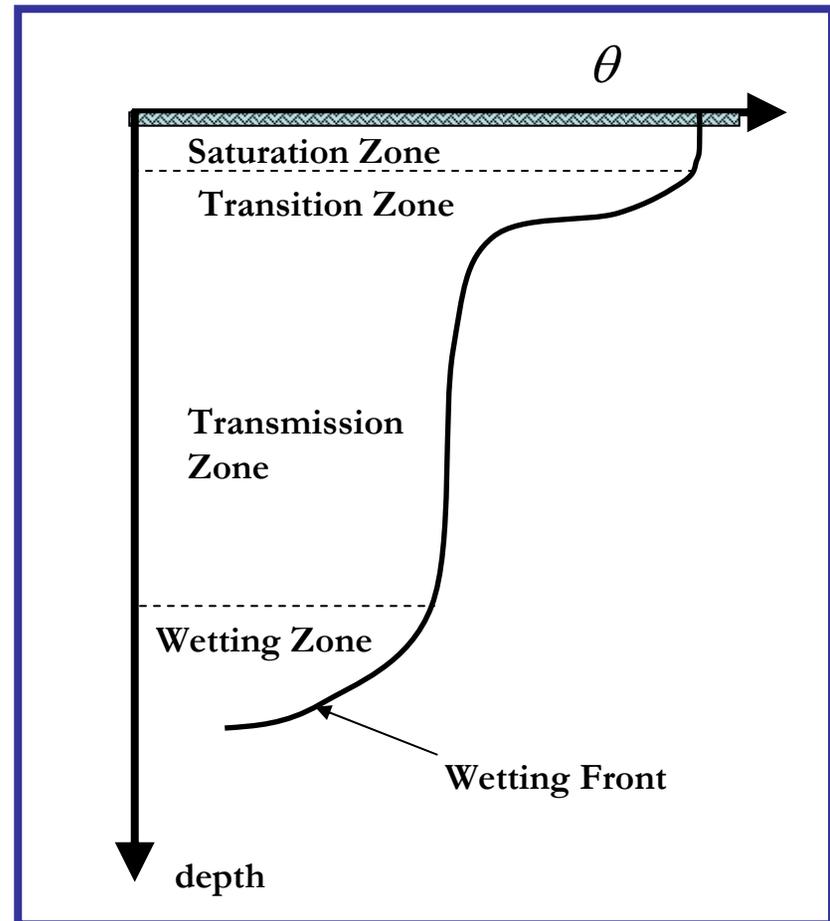
$$\frac{\partial \theta}{\partial t} + \frac{\partial q}{\partial z} = 0$$

$$\frac{\partial \theta}{\partial t} = \frac{\partial}{\partial z} \left(D \frac{\partial \theta}{\partial z} + K \right)$$

Richard's Equation

Infiltration

- General
 - Process of water penetrating from ground into soil
 - Factors affecting
 - Condition of soil surface, vegetative cover, soil properties, hydraulic conductivity, antecedent soil moisture
 - Four zones
 - Saturated zone
 - Transmission zone
 - Wetting zone
 - Wetting front



Infiltration

- Infiltration rate
 - Rate at which water enters the soil at the surface

$$f(t)$$

- Cumulative infiltration
 - Accumulated depth of water infiltrating during given time period

$$F(t) = \int_0^t f(\tau) d\tau$$

$$f(t) = \frac{dF(t)}{dt}$$

Infiltration Methods

- Horton and Phillips
 - Infiltration models developed from
 - approximate solutions to exact theory
(Richard's Equation)
- Green – Ampt
 - Infiltration model developed from
 - exact solution to approximate theory

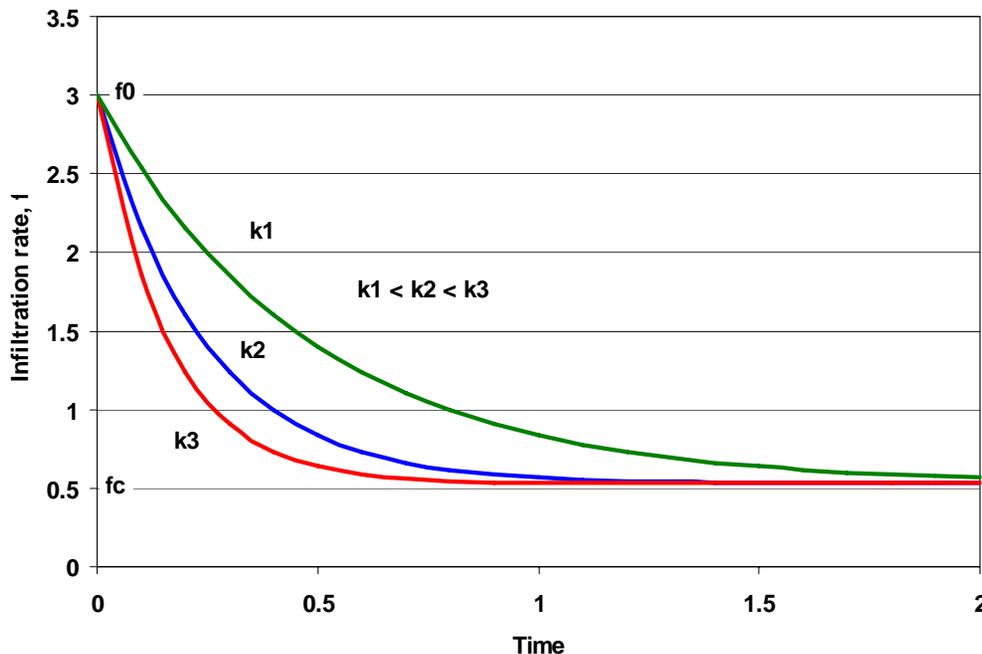
Hortonian Infiltration

- Recall Richard's Equation
 - Assume K and D are constants, not a function of θ or z
- Solve for moisture diffusion at surface

$$\frac{\partial \theta}{\partial t} = \frac{\partial}{\partial z} \left(D \frac{\partial \theta}{\partial z} + K \right)$$

$$\frac{\partial \theta}{\partial t} = D \frac{\partial^2 \theta}{\partial z^2} + \frac{\partial K}{\partial z} \rightarrow 0$$

$$\frac{\partial \theta}{\partial t} = D \frac{\partial^2 \theta}{\partial z^2}$$



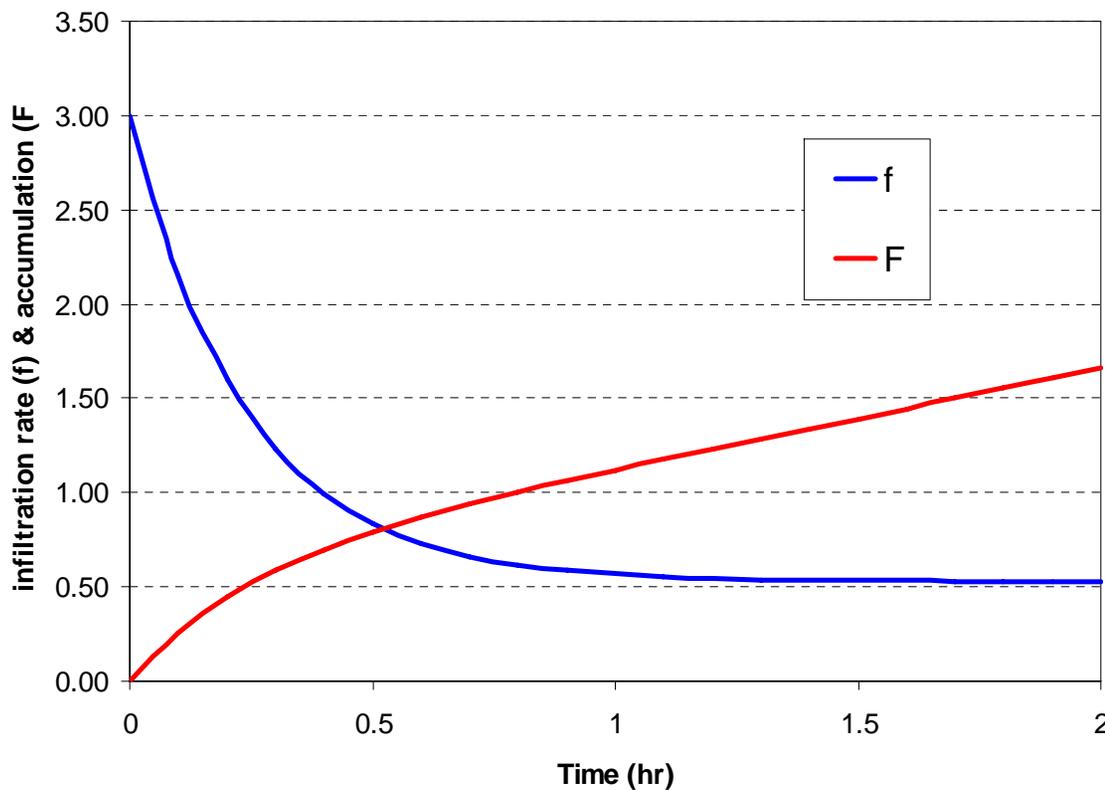
$$f(t) = f_c + (f_0 - f_c)e^{-kt}$$

Horton's Equation

$$F(t) = \int_0^t f(\tau) d\tau$$

Trapezoid Rule

$$F(t_n) = \int_0^{t_n} f(\tau) d\tau \approx \frac{\Delta t}{2} (f_0 + 2f_1 + \dots + 2f_{n-1} + f_n) = \frac{\Delta t}{2} (f_0 + 2 \sum_{i=1}^{n-1} f_i + f_n)$$



t	f	F
0	3.00	0.00
0.1	2.16	0.26
0.2	1.60	0.45
0.3	1.23	0.59
0.4	0.99	0.70
0.5	0.84	0.79
~	~	~
1.8	0.53	1.55
1.9	0.53	1.61
2	0.53	1.66

Philip's Equation

- Recall Richard's Equation

- Assume K and D are functions of θ , not z

$$\frac{\partial \theta}{\partial t} = \frac{\partial}{\partial z} \left(D \frac{\partial \theta}{\partial z} + K \right)$$

- Solution

- Two terms represent effects of
 - Suction head
 - Gravity head

$$f(t) = \frac{1}{2} S \sqrt{t} + Kt$$

Philip's Equation

- S – Sorptivity

- Found from experiment

Green – Ampt Infiltration

L = Depth to Wetting Front

θ_i = Initial Soil Moisture

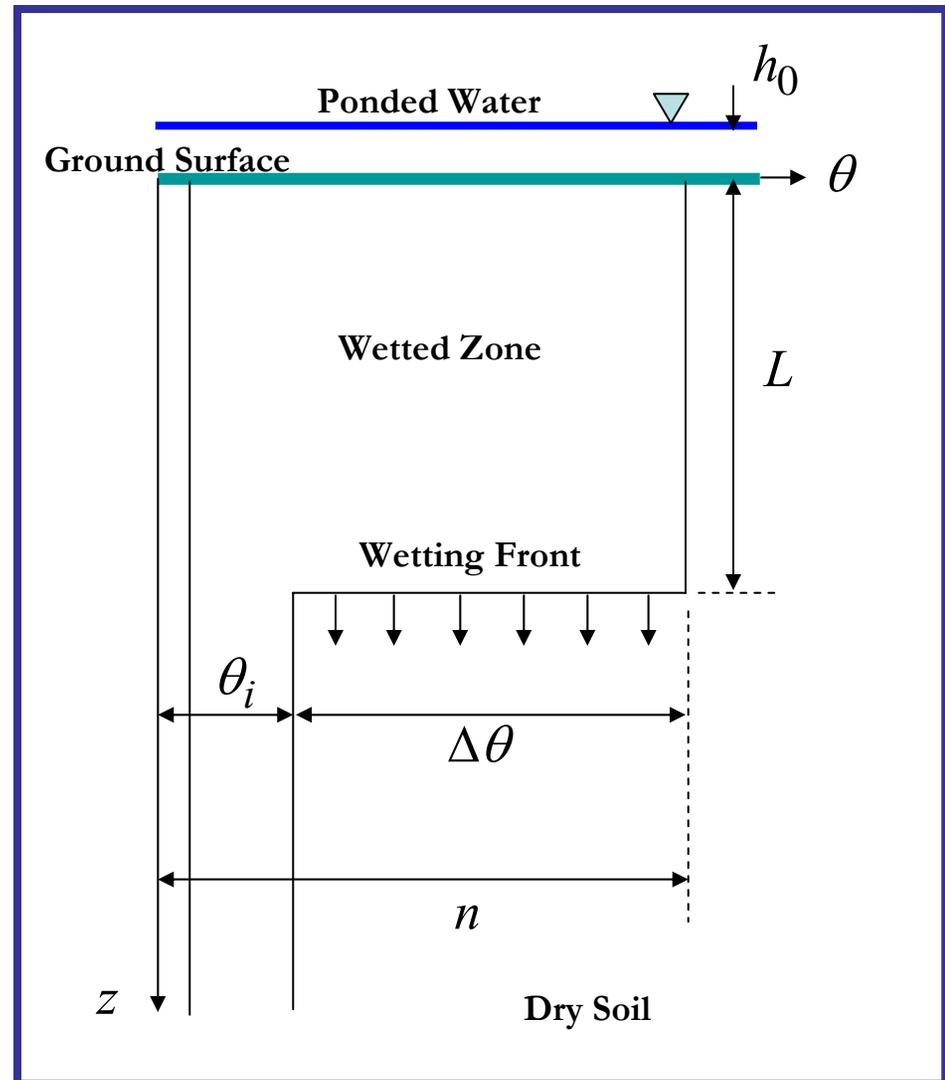
$$F(t) = L(n - \theta_i) = L\Delta\theta$$

$$f = \frac{dF}{dt} = \Delta\theta \frac{dL}{dt}$$

$$q_z = -K \frac{\partial h}{\partial z} = -f$$

$$h = \Psi + z$$

$$f = K \frac{\partial \Psi}{\partial z} + K$$



Green – Ampt Infiltration (Cont.)

$$f = K \frac{\partial \psi}{\partial z} + K$$

- Apply finite difference to the derivative, between

– Ground surface $z = 0, \psi = 0$

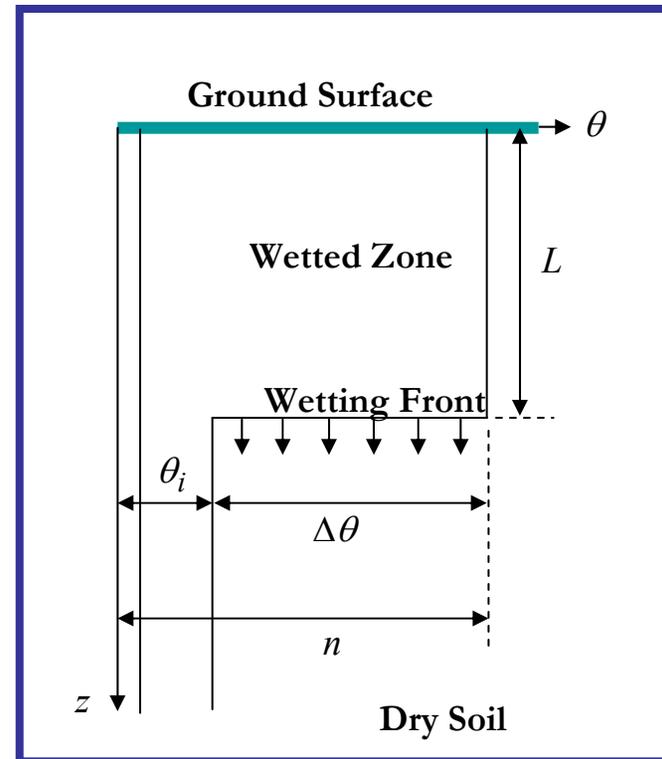
– Wetting front $z = L, \psi = \psi_f$

$$f = K \frac{\partial \psi}{\partial z} + K = K \frac{\Delta \psi}{\Delta z} + K = K \frac{\psi_f - 0}{L - 0} + K$$

$$F(t) = L \Delta \theta$$

$$L = \frac{\Delta \theta}{F}$$

$$f = K \left(\frac{\Delta \theta \psi_f}{F} + 1 \right)$$



Green – Ampt Infiltration (Cont.)

$$f = K \left(\frac{\Delta\theta\psi_f}{F} + 1 \right) \rightarrow f = \Delta\theta \frac{dL}{dt}$$

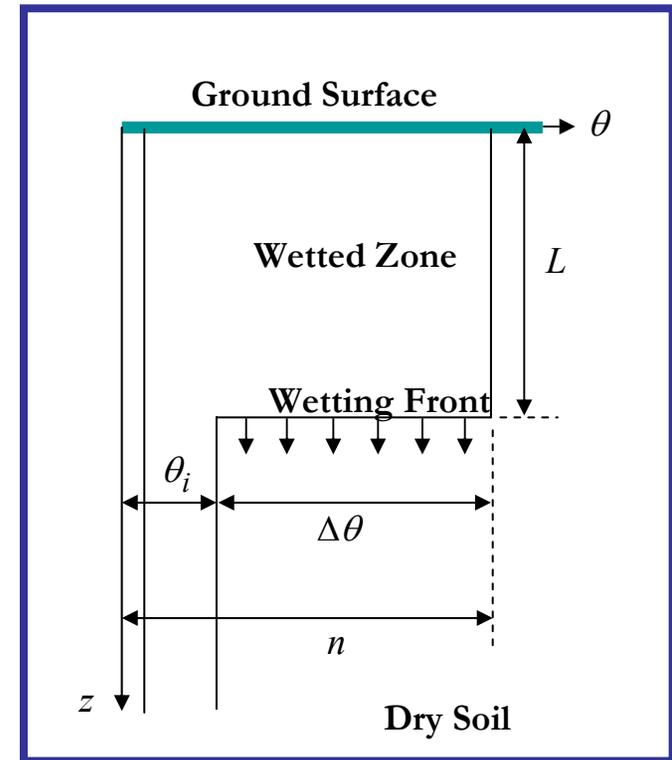
$$F(t) = L\Delta\theta$$

$$\Delta\theta \frac{dL}{dt} = K \left(\frac{\psi_f}{L} + 1 \right)$$

$$\frac{K}{\Delta\theta} dt = dL - \frac{\psi_f dL}{\psi_f + L}$$

Integrate

$$\frac{K}{\Delta\theta} t = L - \psi_f \ln(\psi_f + L) + C$$



Evaluate the constant of integration

$$L = 0 \text{ @ } t = 0$$

$$C = \psi_f \ln(\psi_f + L)$$

$$Kt = L\Delta\theta - \Delta\theta\psi_f \ln\left(\frac{\psi_f}{\psi_f + L}\right)$$

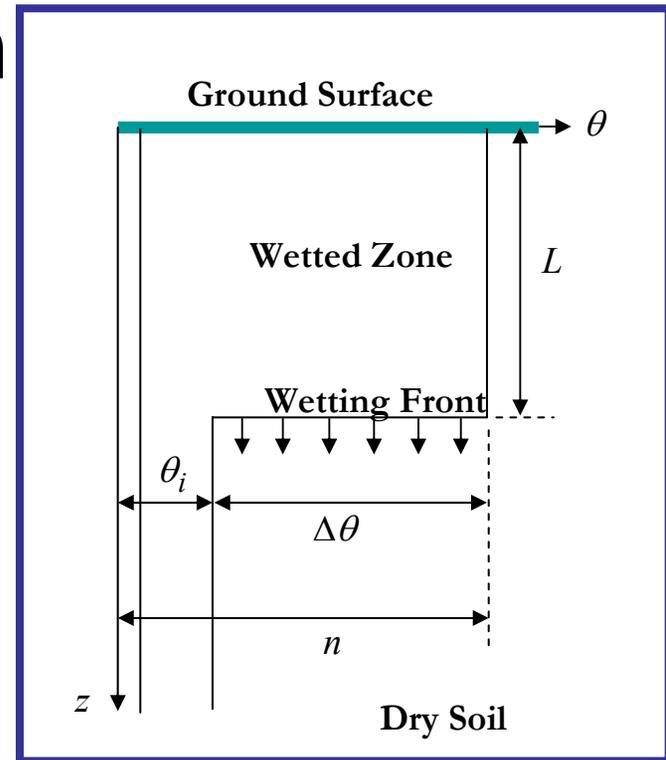
Green – Ampt Infiltration (Cont.)

$$Kt = L\Delta\theta - \Delta\theta\psi_f \ln\left(\frac{\psi_f}{\psi_f + L}\right)$$

$$F = Kt + \Delta\theta\psi_f \ln\left(1 + \frac{F}{\Delta\theta\psi_f}\right)$$

$$f = K\left(\frac{\Delta\theta\psi_f}{F} + 1\right)$$

Nonlinear equation, requiring iterative solution.



See: <http://www.ce.utexas.edu/prof/mckinney/ce311k/Lab/Lab8/Lab8.html>

And: <http://www.ce.utexas.edu/prof/mckinney/ce311k/homework/Solutions-F06/Lab8.pdf>

Soil Parameters

- Green-Ampt model requires
 - Hydraulic conductivity
 - Porosity
 - Wetting Front Suction Head

Soil Class	Porosity	Effective Porosity	Wetting Front Suction Head	Hydraulic Conductivity
	n	θ_e	ψ	K
			(cm)	(cm/h)
Sand	0.437	0.417	4.95	11.78
Loam	0.463	0.434	9.89	0.34
Clay	0.475	0.385	31.63	0.03

$$\theta_e = n - \theta_r \quad \Delta\theta = (1 - s_e)\theta_e \quad s_e = \frac{\theta - \theta_r}{\theta_e}$$

Ponding Time

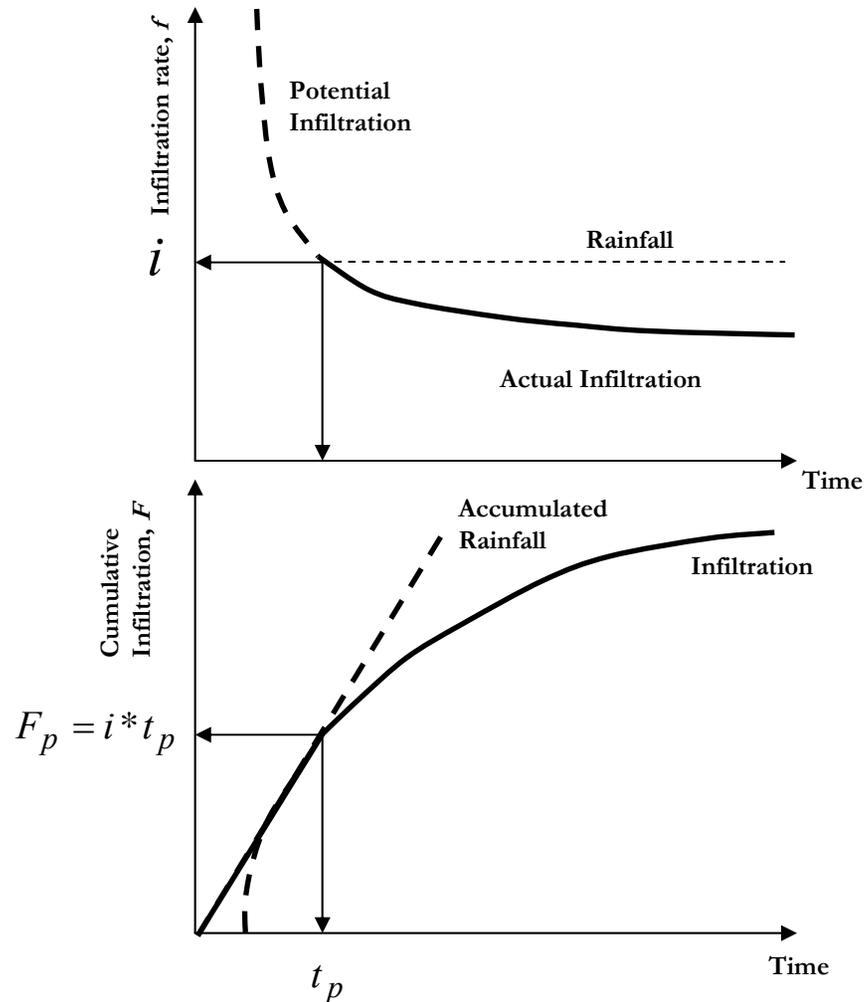
- Up to the time of ponding
 - all rainfall has infiltrated
 - $i =$ rainfall rate

$$f = i \quad F = i * t_p$$

$$f = K \left(\frac{\Delta\theta\psi_f}{F} + 1 \right)$$

$$i = K \left(\frac{\Delta\theta\psi_f}{i * t_p} + 1 \right)$$

$$t_p = K \frac{\Delta\theta\psi_f}{i(i - K)}$$



Example

- Silty-Loam soil
- 30% effective saturation
- 5 cm/hr rainfall intensity

$$\theta_e = 0.486$$

$$\psi = 16.7 \text{ cm}$$

$$K = 0.65 \text{ cm/hr}$$

$$s_e = 0.30$$

$$\Delta\theta = (1 - s_e)\theta_e = (1 - 0.3)(0.486) = 0.340$$

$$\psi\Delta\theta = 16.7 * 0.340$$

$$t_p = K \frac{\Delta\theta\psi_f}{i(i - K)} = 0.65 \frac{5.68}{5.0(5.0 - 0.65)(i - K)} = 0.17 \text{ hr}$$